

I-5 North Coast Corridor San Elijo
Lagoon Improvement Project Proposal

# Cover Letter





1621 Grand Ave. San Diego, CA 92109 (858)-270-3003

November 16th, 2020

Dear CPP Infrastructure Developments,

We are fortunate to have the opportunity to be able to respond to your project proposal. Our team has combined our talents and expertise in creating and planning a design for a proposed new bridge, and we are thrilled to show case our ideas for this proposal. This report includes the proposed design with drawings and calculations as well as the corresponding construction phases and cost estimates.

BB Incorporated has over 50 years of experience in the industry. During the span of our time, our team has delivered numerous successful projects that were under budget and ahead of schedule. BB Incorporated has become one of the most prestigious leaders in the infrastructure community. Our commitment to the project includes the design and reconstruction of I-5 North Coast Highway bridge, overpass bridge, and a pedestrian bridge that are designed to provide decades of use for locals and aesthetic enhancements for the San Elijo Lagoon. Our professional team will exceed project expectations by delivering a final product that is cost effective, time efficient, and of superior quality.

If you have any questions, please contact Project Managers, Matthew Torosian and Jonathan Scott.

Sincerely,

(Matthew's Signature)

Matthew Torosian P.E., S.E.

**Project Manager** 

Jonathan Scott LEED AP

**Project Manager** 

## 2. Summary



### Summary

Established in 1950, BB Incorporated blossomed into an award-winning firm that Design-Build projects. having worked with CPP Infrastructure specializes Developments multiple times before. We have highly qualified staff of talented and experienced professionals with various accomplishments and relationships with high quality subcontractors. The team consists of individuals who have years of experience in both the design and construction of design-build projects, including multiple licensed professional engineers. Our team has worked together on previous projects having accumulated over 10 years of collaboration and trust building. BB Incorporated has an initiative to include environmentally sustainable methods into the design and construction of their projects. We emphasize the constructability of projects which is established by our experience working together for over a decade. BB Incorporated has the team with the experience that can provide innovative solutions for sustainability. BB Incorporated will provide design services that include drawing of the bridge, calculations of the bridge that will be in accordance with GreenRoads and will be continuously involved with the construction process. We also provide construction services that include a site utilization plan, risk mitigation, a safety plan, and quality control plan. We have not had an injury on site in over 3 years. The BB Incorporated team has a cumulative 40 years of relevant research experience in sustainable design and construction methods. We will provide our service for the I-5 North Coast Corridor San Elijo Lagoon Improvement Project for \$203,211,662. BB Incorporated is a leading design build firm in the Southern California region.

# 3. Bonding / Insurance

CPP Infrastructure

3801 West Temple Ave

Pomona, CA 91768



RE: BB Incorporated

I-5 North Coast Corridor San Elijo Lagoon Improvement Project

### Dear CPP Infrastructure:

Travelers Casualty and Surety Company of America, Liberty Mutual Insurance Company, Zurich American Insurance Company and Federal Insurance Company are proud to be associated with BB Incorporated. All four surety companies are licensed to transact surety in all fifty states, including California and are listed as acceptable sureties in the United States Department of Treasury Circular 570 dated July 1, 2020. Over the years, BB Incorporated has developed an impressive resume of completed projects and has a long history of consistent growth and profitability.

Given the level of comfort the sureties have developed with BB Incorporated technical, financial, and managerial resources, the sureties have established a surety line of credit of \$900,000,000 for single projects, subject to an aggregate uncompleted backing of \$9,000,000,000. The current available bonding capacity is in excess of \$2,000,000,000.

It is our intent to provide performance and payment bonds for the above referenced project in the event BB Incorporated is awarded the project. Our intent is conditioned upon applicable underwriting considerations such as: contract terms, bond forms, confirmation of project financing and any other underwriting considerations at the time of project award. This letter is not an assumption of liability, nor is it a bid bond or a performance bond. It is issued as a bonding reference by us for our client. If you should need an addition information, please do not hesitate to contact our office.

Sincerely.

Travelers Casualty and Surety Company of America (AM Best Rating A++, XV)

Liberty Mutual Insurance Company (AM Best Rating A, XV)

Zurich American Insurance Company (AM Best Rating A+, XV)

Federal Insurance Company (AM Best Rating A++, XV)

Mark Andreus

Mark Andrews

Attorney-In-Fact

November 10, 2020



3801 W Temple Ave, Pomona, CA 91768



RE: I-5 North Coast Corridor San Elijo Lagoon Improvement Project

To Whom It May Concern,

Alliant Insurance Services, Inc. is the insurance company for BB Incorporated. BB Incorporated is able to obtain insurance coverage in the required limits and ratings required for this project. Coverage can be provided using current practice policies and/or through the utilization of project specific policies to meet the insurance requirements found in Section IV- Insurance Requirements. Insurance coverage can be issued by insurance companies licensed to conduct such business under the laws of the State of California with an AM Best rating of at least A- or better.

Please do not hesitate to contact our office should you have any questions or if we may be of additional service.

Sincerely,

Mary Nin

Account Manager

**Construction Services Group** 

Phone: 626-660-6413



### **Certificate of Insurance**

DATE

11/09/2020

This certificate is issued as a matter of information only and confers no rights upon certificate holder. This does not affirmatively or negatively amend, extend, or alter the coverage afforded by the policies below. This certificate of insurance does not constitute a contract between the issuing insurer(s), authorized representative or producer, and the certificate holder.

Producer Los Angeles - Alliant Insurance Services, Inc. 333 S Hope St Ste 3750 Los Angeles CA 90071	Phone 626-660-6412	
Insured	Insurer Affording Coverage	NAJC
BB Incoporated	Insurer A: Zurich American Insurance Corp	18673
1621 Grand Ave	Insurer B: American Zurich Insurance Comp	46362
San Diego CA 92109		

Certificate Number: 238362910

This is to certify that the policies of insurance listed below have been issued to the insured named above for the policy period indicated. Notwithstanding any requirement, term or condition of any contract or other documents with respect to which this certificate may be issued or may pertain. The insurance afforded by the policies described here is subject to all the terms, exclusions, and conditions of such policies.

Type of insurance	Additional Insured	Policy Number	Policy Exp.	Limits			
Comercial General Liability	CPP Infrastructure	GLO727384919	11/18/2021	Occurrence	\$10,000,000	Aggregat e	\$10,000,000
Auto Liability	CPP Infrastructure	BAP272383291	11/18/2021	Occurrence	\$5,000,000	Aggregat e	\$1,000,000
Professional Liability		TYK172830178	11/18/2021	Occurrence	\$5,000,000	Aggregat e	\$10,000,000
Excess Umbrella Liability	CPP Infrastructure	XOOG5647896Y224	11/18/2021	Occurrence	\$5,000,000	Aggregat e	\$5,000,000
Workers Compensation & Employers Liability		WC567895434	11/18/2021	E.L. Each Accident	\$1,000,000	E.L. Disease EA	\$1,000,000

Waiver of subrogation endorsement: CPP Infrastructure is included in all policies above.

Certificate Holder	Representative Signature
BB Incoporated	44 . ~ .
1621 Grand Ave	Martin Tewie
San Diego CA 92109	Trial ciri Tobbic

## 4. Safety Record, Claims, Disputes, and/or OSHA Violations



**RE: Experience Modification Rating** 

To Whom It May Concern:

Alliant Insurance Services Inc. is the Insurance Agent for BB Incorporated. This letter serves to confirm your Experience Modification Rating for the following years.

<u>Year</u>	Modification Rating
2019:	.58
2018:	.57
2017:	.66
2016:	.63
2015:	.59

Please contact us if you have any further questions.

Sincerely,

Mary Nin

Account Manager

**Construction Services Group** 

Phone: 626-660-6413

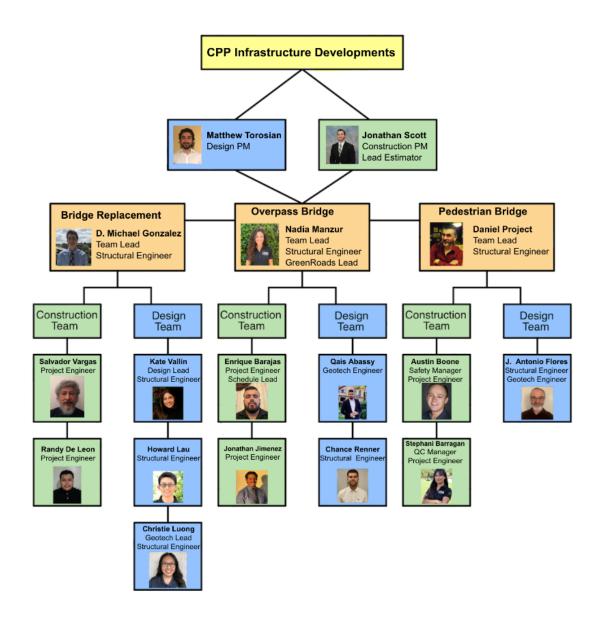
### Litigation, Claims, and Disputes History

BB Incorporated has NO litigations on any projects in the past 5 years. Further BB Incorporated has had NO disputes or claims on any project in the past 5 years related to poor workmanship, incomplete performance or defective work, not honoring warranty items during contract warranty period, or unexcused delays in completion asserted by owner or bonding surety against contractors.

### **OSHA** Violations

BB Incorporated has had no OSHA violations in the past five years.

# 5.Organization and Staffing



### **Organization Chart**

Our team's organization chart shows the best positions of our structural and construction engineers that makes every project seemingly flow. This organization chart shows which members have expertise in different parts of the project and we feel that this organization helps our firm work and communicate better.

### **Team History**

At BB Incorporated, our team players have been working together for over 10 years. Every project that our company has worked on always become success stories and this project will be known as one of them as well. PMs Matthew Torosian and Jonathan Scott worked on various projects together, one being the Gerald Desmond Replacement Bridge in 2013 to 2020. Both Matthew and Jonathan worked so well together, they decided to open up their own design build firm named BB Incorporated making sure to bring along engineers that have worked with each other as well. Most engineers that are associated with this firm have also worked on the Gerald Desmond Replacement Bridge. This project showed how collaborative and creative our team is and how we can design and build in a way that is time efficient and cost effective for our clients. This project is very similar to the San Elijo Lagoon Project, therefore we believe that our team is confident in providing the best and most efficient designs possible.

### **Project Commitment**

Our team has carefully designed and planned how to proceed with this project if the proposal is accepted. The project will need constant attention and continuous collaboration from both the design and construction members. We are committed to delivering this project on schedule and at the given budget. We will achieve this by cycling through the same pool of selected construction members to have on the job site at all times. This will help with management of the project which will help finish the project on time. We are also committed to have selected design members to have site visits at least once a week throughout the construction process. This is to address any concerns or any problems that can not be anticipated for. Doing this will assure a safe and timely delivery of the project. Safety is our number one priority and we plan to make sure that staff and engineers are safe at all times and wearing the proper field gear. Our risk manager carefully planned out on how to avoid all the risk and hazards that may occur during the construction of the project to ensure that these situations do not become fatal.

### Jonathan Scott

Role on Project: Construction Project Manager / Lead

Estimator

### **Areas of Expertise:**

- Design Build Delivery Method
- Bridge Construction
- Safety
- Cost Control



Jonathan Scott will be the Construction Project Manager on the project. He has over 35 years of construction industry experience, 20 of which being Project Manager. Mr. Scott is involved in all engineering decisions and is responsible for the day to day management of projects from initial planning stages to final construction support services.

### **Relevant Project:**

- <u>2013 2020 Gerald Desmond Replacement Bridge: Construction Project Manager</u>
  - Long Beach, California
  - o Cable Stayed Bridge
  - o Design Build
  - o \$1.5 Billion
- 2004 2011 Interstate 15 Managed Lanes: Construction Project Manager
  - San Diego, California
  - o Managed Lanes, and Highway Widening
  - o \$420 Million
- 1999 2004 Sundial Bridge: Construction Project Manager
  - Redding, California
  - Cable Stayed Pedestrian Bridge
  - Design Build
  - \$23.5 Million

### **Employment:**

1990 - 2020: BB Incorporated
 1985 - 1990: Kiewit Construction

### Licenses:

• LEED AP

### **Education:**

BS, Construction Engineering Technology California State Polytechnic University, Pomona, 1985



Matthew Torosian P.E., S.E.

Role on Project: Design Project Manager

### **Areas of Expertise:**

- Highway Design Build
- Structural Engineering

### **Qualifications:**

Matthew Torosian has over 35 years of design experience with 15 years of design management experience. He has worked on various bridge designs and has managed all aspects of design from the substructure to the superstructure.

### **Relevant Project:**

- Gerald Desmond Replacement Bridge: Design Project Manager
  - o Long Beach, California
  - Cable Stayed Bridge
  - o Design Build
  - o \$1.5 Billion
- Bayonne Bridge: Lead Designer
  - New Jersey
  - Tied-Arch Bridge
  - Bridge Inspection and Rehabilitation
  - o \$1.3 Billion

### **Employment:**

- 1992 Present **BB Incorporated**
- 1987 1992 KPFF Consulting Engineers
- 1982 1987 **DCI Engineers**

### **Registrations:**

Registered Professional Engineer, California (#123456) Registered Structural Engineer, California (#7890)

### **Education:**

MS, Structural Engineering, University of California, San Diego BS, Civil Engineering, California State Polytechnic University, Pomona



David Michael Gonzalez S.E, P.E.

**Role on Project:** Structural Engineer / Main Bridge Lead Engineer

### **Areas of Expertise:**

- Design Build Delivery Method
- Highway Bridge Design
- Structural Engineering

### **Oualifications:**

Michael Gonzalez will be the Main Replacement Bridge Lead on the project.

### **Relevant Project:**

- 2011-2015 Brooklyn Bridge repair: Design Project Manager
  - New York City, New York
  - Suspension Bridge
  - o Complete redesign, refurbishing and reinforcement or strength of bridge.
  - \$33.5 Million
- 1998 2001 Bronx-Whitestone Bridge repair: Structural Engineer
  - New York City, New York
  - Suspension Bridge
  - Design Build
  - \$286 Million

### **Employment:**

- 2015 Present: **BB Incorporated**
- 2006 2015: **NYCDOT**
- 1998 2006: MTA Bridges and Tunnels
- 1994 1998: **Kimley Horn**
- 1992 1994: Transportation and Energy Solutions

### Registration:

Registered Professional Engineer, California, New York Registered Structural Engineer, California, New York

### **Education:**

B.S. Civil Engineering, California State Polytechnic University, Pomona, 1987 M.S. Structural Engineering, California State Polytechnic University, Pomona, 1990 PHD Structural Engineering, UC Berkeley, 1992



Howard Lau, P.E., S.E.

**Role on Project:** Structural Engineer

### **Areas of Expertise:**

• Highway Bridge Design

### **Qualifications:**

Howard Lau will be a structural engineer for the replacement bridge. From his 25 years of working in the bridge engineering industry, Howard has amassed a stellar track-record with his

specialties entailing cable stayed bridge design, arch bridge design, and highway bridge expansions.



- 2013 2020 Gordie Howe International Bridge: Principal Engineer
  - O Detroit, Michigan
  - o International Cable Stayed Bridge
  - Design Build
  - \$5.7 Billion
- 2002 2008 I-5/I-805/SR56 Reconstruction: Design Engineer
  - o San Diego, California
  - Highway Widening and New Freeway Bypasses
  - \$120 Million
- 1999 2004 Sundial Bridge: Assistant Design Engineer
  - o Redding, California
  - Cable Stayed Pedestrian Bridge
  - Design Build
  - \$23.5 Million

### **Employment:**

• 1995 - Present: **BB Incorporated** 

### **Registration:**

- Professional Engineer, CA
- Structural Engineer, CA

### **Education:**

BS, Civil Engineering MS, Structural Engineering California State Polytechnic University, Pomona, 1994



Kate Vallin P.E., S.E., PhD

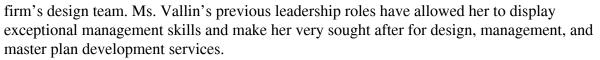
Role on Project: Structural Engineer, Design Lead

### **Areas of Expertise:**

- Structural Engineering
- Seismic Engineering

### **Qualifications:**

Kate Vallin, a structural engineer on the project contributes 25 years of experience in structural design for successful public and private projects throughout California. Her background in seismic performance of elevated bridges have proven to be vital to the



### **Relevant Project:**

- Gerald Desmond Replacement Bridge: Structural Engineer 2013-2020
  - o Long Beach, California
  - o Cable Stayed Bridge
  - o \$1.5 Billion
- Interstate 15 Managed Lanes: Structural Engineer 2004-2011
  - o San Diego, CA
  - o Highway Lane Expansion
  - o \$420 million

### **Employment:**

- 2003-Present **BB Incorporated**
- 2002-2013 **Flatiron**
- 1981-2001 Weldinger Associates

### **Registrations:**

Professional Engineer , California Structural Engineer , California

### **Accomplishments:**

ASCE Outstanding Civil Engineer Advocate of the Year Award- Individual 2014 ASCE Planning & Management Council Services to the Profession Award-2019

### **Education:**

BS, Civil Engineering, 1989 California State Polytechnic University, Pomona MS, Structural Engineering, 1994 University of California, Los Angeles PhD, Seismic Structural Engineering 2000 University of California, San Diego



Christie Luong PE, SE, LEED AP

Role on Project: Structural Engineer/ Lead Geotech Engineer

### **Areas of Expertise:**

- Design Build Delivery Method
- Bridge Design
- Structural Engineering
- Geotechnical Engineering



Christie Luong will be the Structural Engineer and the lead

Geotech Engineer on the project. She has over 27 years of experience in bridge design and projects, 17 of which is being a certified geotechnical engineer and 10 and as a licensed structural engineer. Christie is involved in all geotechnical decisions and never fails to provide the best and most efficient designs possible.

### **Relevant Project:**

- <u>2013 2020 Gerald Desmond Replacement Bridge: Estimator</u>
  - OLong Beach, California
  - Cable Stayed Bridge
  - o Design Build
  - \$1.5 Billion
- 2003 2013 Oakland Bay Bridge
  - Oakland, California
  - Demolition and Reconstruction of old bridge
  - \$6.4 Billion
- State Highway 71 Express Lanes
  - o Austin, Texas
  - o Improvements of a 4-mile corridor
  - New construction of overpass bridges.
  - \$93 Million

### **Employment:**

- 2008 Present: **BB Incorporated**
- 1998 2008: McCarthy Construction Company
- 1991 1997: **Dewberry**
- 1989-1991: **City of Oakland**

### **Education:**

- B.S. General Civil Engineering Cal Poly Pomona (1991)
- M.S. Structural Engineering UC Berkeley (1993)



### Randy De Leon, PE MCM

### **Role on Project** Project Engineer

### **Area(s) of Expertise:**

- Estimating
- Scheduling
- Take offs
- Safety Management

### **Qualifications:**

Randy De Leon will be the Construction Project engineer on the main bridge replacement. He has over 25 years of construction

experience in the field. 15 years of being a project engineer. Mr. De Leon is involved in all Construction project engineering decisions for the bridge replacement such as estimating, traffic management, scheduling, and safety plan, Safety Coordinator.

### **Relevant Project:**

- 2010-2017 Gerald Desmond replacement bridge: Construction Project Engineer
  - o Long Beach, California
  - o Cable Stayed Bridge
  - o Design Build
  - o 1.5 Billion
- 2000-2009 Interstate 15 Manage Lanes: Construction Project Engineer
  - o San Diego
  - o Manage Lanes, and Highway
  - o \$42 Million

### **Employment:**

1985-2020: BB Incorporated
 1975-1985: Traylor Bro

### License:

• PLS, PE

### Education

MCM in Construction Management, University of Southern California B.S. in Civil Engineering, California Polytechnic University A. A General Science, Santa Monica College



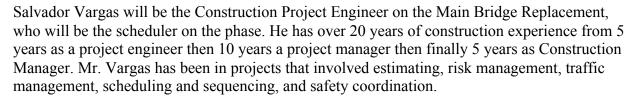
Salvador Vargas P.E., PLS, LEED AP

Role on Project: Project Engineer

### **Area(s) of Expertise:**

- Scheduling
- Sequencing
- Estimating,
- Take offs
- Safety management

### **Qualifications:**



### **Relevant Projects:**

- 2010-2017 Gerald Desmond replacement bridge: Construction Project Engineer
  - o Long Beach, California
  - Cable Stayed Bridge
  - Design Build
  - o 1.5 Billion
- 2000-2009 Interstate 15 Manage Lanes: Construction Project Engineer
  - o San Diego, California
  - Manage Lanes, and Highway Widening
  - o \$420 Million

### **Employment:**

• 2010-2020: **BB Incorporated** 

• 2005-2010: **PCL** 

• 2000-2005: **Traylor Bros, Inc.** 

### **Registrations:**

Professional Engineer , California LEED AP PLS

### **Education:**

BS in Civil Engineering, General from CPP

MS Structural Engineering & MS in Geotechnical Engineering from CPP & CSU Long Beach



Nadia Manzur P.E., T.E

Role of Project: Green Road and Structural Engineer

### **Areas of Expertise:**

- Green Roads Engineering
- Structural Engineering

### **Qualifications:**

Nadia Manzur is the Structural Engineer for the Overpass Bridges and Green Road Lead for the project. With her 20 years in the construction and engineering industry, she will be able to help the team achieve a successful project by going green.



### **Relevant Projects:**

- <u>I-15 Tech Corridor Expansion</u>
  - o Lehi, Utah
  - o Adding lanes to a highway
  - o Total of 6 bridge project
- Gerald Desmond Bridge: Replacement Bridge
  - o Long Beach, California
  - o Replacement of a bridge to improve traffic flow
- <u>I-5/ Genesee Avenue Interchange</u>
  - o San Diego, California
  - o Replace five-lanes with a 10-lane overcrossing

### **Employment:**

2006-Present: BB Incorporated
1999-2006: Ames Construction

### **Registrations**:

- Professional Engineer, CA
- Traffic Engineer, CA

### **Education:**

BS, Civil Engineering

California State Polytechnic University, Pomona, 2000

Qais Abassy, P.E, LSP

**Role on Project:** Structural Engineer / Geotechnical Engineer

### **Areas of Expertise:**

- Design-Build
- Structural Engineering
- Bridge Construction

### **Qualifications:**

Qais Abassy, a structural engineer on the project, contributes. 20 years of experience in bridge construction and design for many successful projects in California. His knowledge in

designing bridges and in construction has served BB Incorporated well in his 20 years of experience.



- SR-91 Riverside Freeway
  - o Riverside, California
  - o Added a lane on SR-91 between SR-241 and I-15
- Gerald Desmond Bridge: Replacement Bridge
  - o Long Beach, California
  - o Replacement of a bridge to improve traffic flow
- <u>SR 57 Northbound Widening Project</u>
  - o Orange County, California
  - o Added an additional northbound lane on the Orange Freeway (SR-57)
  - o Widened all existing NB lanes and the NB median shoulder

### **Employment:**

• 1997 - 2005: Alabbasi Constructions and Engineering

• 2005 - Present: **BB Incorporated** 

### **Licenses:**

Professional Engineer, California

### **Education:**

BS, Civil Engineering

California State Polytechnic University, Pomona, 1997



Chance J. Renner P.E, S.E.

Role On Project: Structural Engineer

### **Areas of Expertise:**

- Design Build Delivery Method
- Bridge Design
- Structural Engineering

**Qualifications:** As the structural engineer, I will be responsible for the overpass portion of the project. This includes design oversight, and structural design, etc. I will coordinate work between the production team and the client, and submit all deliverables in a timely manner. I ensure that the design, planning, and



specifications produced by BB Incorporated will continue to exceed industry standards.

### **Relevant Projects:**

- SR-91 Riverside Freeway
  - o Riverside, California
  - o Added a lane on SR-91 between SR-241 and I-15
- Gerald Desmond Bridge: Replacement Bridge
  - o Long Beach, California
  - o Replacement of a bridge to improve traffic flow
- SR 57 Northbound Widening Project
  - o Orange County, California
  - o Added an additional northbound lane on the Orange Freeway (SR-57)
  - o Widened all existing NB lanes and the NB median shoulder

### **Employment:**

• 2008 - Present: **BB Incorporated** 

• 1998 - 2008: Michael Baker International

### **Registration:**

Structural Engineer, California Professional Engineer, California

### **Education:**

BS, Civil Engineering, California State Polytechnic University, Pomona, 1997

Enrique Barajas, P.E., QSP, QSD

**Role on Project:** Schedule Manager / Project Engineer

### **Areas of Expertise:**

- Managing
- Design-Build
- Construction Scheduling

### **Qualifications:**

Enrique Barajas serves as a Schedule Manager at BB Incorporated. His knowledge of construction materials, construction work, experience and operations has served BB Incorporated well in his over 30 years of construction industry experience.

### **Relevant Projects:**

- <u>07-4w8204 Camarillo, California</u>
  - o Girder Replacement on the 101 in Camarillo
- Rancho Santa Margarita, California
  - Hinge Replacement
- NewPort Beach, California
  - Bridge Work Adding Lanes to a Bridge

### **Employment:**

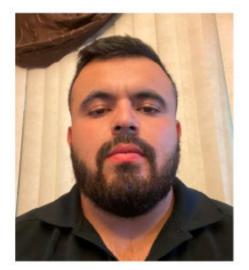
- 1996 Present: **BB Incorporated**
- 1986 1996: **Beador Construction Company**

### **Licenses:**

Professional Engineer, QSP / QSD

### **Education:**

BS, Civil Engineering, Cal Poly Pomona, 1986 Masters, Construction Management, USC,1988



### Jonathan Jimenez

Role on Project: Project Engineer

### **Areas of Expertise:**

- Design Build Delivery Method
- Transportation Estimating
- Project Engineering
- Cost Analysis

### **Qualifications:**

Jonathan Jimenez will be the project engineer for the overpass portion on the project. Jonathan has 19 years of construction industry experience. 4 years as a project engineer and 15 years of estimating. Mr. Jimenez has been one of our top estimators at BB incorporated. His experience with multiple transportation related projects makes Mr. Jimenez a perfect candidate for this position. Jonathan

Jimenez will be a great asset to this project and will add a large amount of expertise to the team.

### **Relevant Project:**

- 2013 2020 Gerald Desmond Replacement Bridge: Estimator
  - ○Long Beach, California
  - Cable Stayed Bridge
  - Design Build
  - \$1.5 Billion
- 2005 2008 Surf City Replacement Bridge: Estimator
  - Surf City, North Carolina
  - o Demolition and replacement bridge
  - \$700 Million
- 2001 2004 I-15 Interstate: Project Engineer
  - o Riverside, California
  - Additions to interchange; overpasses and ramps
  - Design Build
  - \$300 Million

### **Employment:**

- 2008 2020: **BB Incorporated**
- 2004 2008: Skanska USA
- 2001 2004: Shimmick Construction

### **Education:**

BS, Civil Engineering

California State Polytechnic University, Pomona, 2001



Daniel Sandoval PE, SE

Role on Project: Pedestrian Bridge Team Lead

### **Areas of Expertise:**

- Structural Design
- Bridge Design
- Seismic Design

### **Oualifications:**

Daniel Sandoval will be the Pedestrian Bridge Team Lead

With 25 years of professional civil and structural engineering

experience, our project expertise covers Structural Engineering, Reinforced concrete design, Bridge Design and Seismic Design. Sandoval will be responsible for the day to day operation of the Pedestrian Bridge team as well as work closely with the other teams to ensure efficiency and the best possible outcome..

- 2014-2019 Kosciuszko Bridge: Design Manager
  - Newton Creek , New York
  - Cable Stayed Bridge
  - Design Build
  - \$555 Million
- 2005-2008 Bob Kerrey Pedestrian Bridge: Design Manager
  - Omaha, Nebraska
  - Cable Stayed Bridge
  - \$22 Million
- 1999 2004 Sundial Bridge: Structural Design Lead
  - o Redding, California
  - Cable Stayed Pedestrian Bridge
  - Design Build
  - \$23.5 Million

### **Employment:**

- 1995-2003 Burns and Mcdonnell
- 2003-2012 **Kiewitt**
- 2012-2019 **HNTB Cos**.
- 2019 Present **BB Incorporated**

### **Licenses:**

Structural Engineer, California Professional Engineer, California

### **Education:**

BS, Civil Engineering, California State Polytechnic University, Pomona, 1993 MS, Structural Engineering, University of California San Diego, 1995



Jose Antonio Flores Apaez P.E

**Project Role**: Structural Engineer and Geotechnical Engineer.

### **Areas of Expertise:**

- Bridge Design
- Geotechnical Engineering
- Structural Engineering

### **Qualifications:**

Jose Antonio Flores Apaez will be the Geotechnical Engineer in charge of the design of the foundation of the pedestrian bridge and

retaining walls along SD-5. He has 30 years of bridge design experience with 20 of those years have been geotechnical design. He has been with BB cooperated for the past 15 years and always gets the projects done smoothly. Mr. Flores will work endlessly with the design team and construction team to compute a foundation design that is feasible, innovative and cost friendly for the proposed bridge and retaining walls.

### **Relevant Projects:**

- <u>Dublin Link Bridge: Bridge Designer:</u>
  - o Dublin, Ohio
  - o Cable Stayed Pedestrian Bridge
  - o Main Towers provide no vertical Support
  - o Design Build Project
  - \$23 Million
- Chesapeake Bay Bridge: Geotechnical Engineer
  - Best Project Award
  - National Honor Award from ASEC
  - Cable Design of Dehumidification system for Bay Bridge
- 1999 2004 Sundial Bridge: Geotechnical Engineer
  - o Redding, California
  - Cable Stayed Pedestrian Bridge
  - o Design Build
  - \$23.5 Million

### **Employment:**

- 1995-Present **BB Incorporated**
- 1985-1995 T.Y. International Group
- 1980-1985 **Stantec**

### **Registration:**

Professional Engineer, CA

### **Education:**

BS, Civil Engineering California Polytechnic State University. MS in Geotechnical Engineering from UCSD



Austin Boone, P.E., QSP, LSP

Role on Project: Project Safety Manager / Project Engineer

### **Areas of Expertise:**

- Safety
- Design-Build
- Construction Cost Estimating and Tracking

### **Qualifications:**

Austin Boone serves as a project engineer as well as a project safety manager at BB Incorporated. His knowledge of construction materials and operations, combined with his extensive emphasis and passion for safety, has served BB Incorporated well in his over 30 years of construction industry experience.



### **Relevant Projects:**

- <u>2013 2020 Gerald Desmond Replacement Bridge: Construction Pro</u>
  - Norwalk, CA
  - I-5 Widening and HOV Project
  - \$323 Million
- 2004 2011 Interstate 15 Managed Lanes: Construction Project Manager
  - San Diego, California
  - Managed Lanes, and Highway Widening
  - \$420 Million
- 1999 2004 Sundial Bridge: Construction Project Manager
  - Redding, California
  - Cable Stayed Pedestrian Bridge
  - Design-Build
  - \$23.5 Million

### **Employment:**

- 1994 Present: **BB Incorporated**
- 1987 1994: **The Walsh Group**

### **Licenses:**

Licensed Safety Professional, P.E.

**Education:** BS, Civil Engineering, Cal Poly Pomona, 1987

### Stephani Barragan

Project Role: Project Engineer & Quality Control Manager.

### **Areas of Expertise:**

- Design Build Delivery Method
- Scheduling
- Quality Control
- Project Engineering

### **Qualifications:**

Stephani Barragan has over 15 years of experience in project scheduling working on projects throughout California. Project scheduler oversees the construction of a project making sure that all deadlines are met. Her

ability of creating great relationships with developers and subcontractors and collaborating with a variety of clients, engineers, designers to successfully schedule projects from beginning to end is so uniquely that has achieved awards.



2011-2020 Gerald Desmond Replacement Bridge: Scheduler

- OLong Beach, California
- Cable Stayed Bridge
- o Design Build
- \$1.5 Billion
- 2005-2011 Interstate 15 Managed Lanes: Project Engineer
  - o San Diego, California
  - o Bid Build Delivery
  - o \$420 million
  - New 20-mile section of High Occupancy Toll (HOT) lanes stretching from State Route
     163 north to State Route 78 along Interstate 15 in San Diego, Calif.
- 1998-2005 Sundial Bridge: Project Engineer
  - o Redding, California
  - Cable Stayed Pedestrian Bridge
  - Design Build
  - o \$23.5 Million

### **Employment:**

2011-Present: BB Incorporated

2005-2011: Flatiron

1998-2005: Burns and Mcdonnell

**Licenses:** 

Professional Engineer, CA

### **Education:**

BS, Civil Engineering California Polytechnic State University.



# 6. Management Systems

### Cost & Schedule Control

BB Incorporated will implement a number of different methods to control the cost and schedule of the project. BB Incorporated's highly certified project engineers will track costs for all trades including labor, material, and equipment. They will also review all change orders to ensure sub-contractor's costs are not inflated. The change order will then be presented to the Project Manager for final approval before being sent to the owner. BB Incorporated will also include a \$200,000,000 GMP for the project. BB incorporated will be utilizing the Critical Path Method to control the schedule. With this method BB Incorporated's schedulers will be able to identify the most critical activities. Once these activities are identified they will be monitored closely and if necessary, these activities can be accelerated to meet the schedule requirements. The project will also incorporate earned value method. This method allows the Project Manager to measure actual work performed for an activity. With this he can predict the total cost of an activity and it's completion date.

## 7. Self-Perform Work

### Self-Perform Work

BB Incorporated's own highly qualified carpenters, laborers, and cement masons can self-perform traditional general contractor work such as soft demolition, structural excavation, concrete formwork, and concrete placement and finishing. While it is not essential, the ability to self-perform work can greatly benefit the overall project. Advantages include:

**Schedule**: Self-performed work greatly increases BB Incorporated ability to aggressively manage and drive the schedule

**Cost**: Higher subcontractor mark-ups for overhead and profit are saved on self-perform work, and efficiencies can be gained with consolidated BB Incorporated crews and supervision.

Quality: BB Incorporated has direct control over the quality of work performed.

**Safety**: BB Incorporated knows that with self-performed work, safety is always at the forefront with our highly trained field personnel. And more BB Incorporated personnel working in the field equates a safer project – more BB Incorporated "eyes" on the project, sharing and enforcing our culture of safety with all subcontractor trades.

**Project Control**: With the increased levels of schedule, cost, quality, and safety management, BB Incorporated has a better ability to control the overall project.

# 8. Relevant Experience

## **Related Projects**

#### Gerald Desmond Bridge 2013-2020

Type: Cable Stayed Highway Bridge

Size: 8,800 feet Cost: \$1.1 Billion

Scope of Work: Replacement of the Previous Gerald Desmond Bridge

**Key Features** 

Cable Stayed Bridge

Constructed over a body of water

Owner Information

Phone #: (916) 654-5266



### Imperial Highway / Pioneer Blvd. Project 2013-2019

Type: Highway Bridge Widening and Bridge Reconstruction

Size: 1.89 Miles Cost: \$323 Million

Scope of Work: Widened two miles of I-5 to add one High Occupancy Vehicle Lane (HOV) or carpool lane and one general purpose lane in each direction. Three bridges were reconstructed to accommodate the widened freeway. The project included shoulder widening, new ramp alignment, concrete median barriers, sound walls, and decorative tile work.

#### Key Features

- enhance safety
- add traffic lanes
- encourage ride sharing through new HOV lanes
- decrease surface street traffic
- help improve air quality

Owner Information:

#### Phone #: (916) 654-5266



## Rosecrans Ave. / Bloomfield Ave. Bridge Project 2012-2017

Type: Highway Bridge Widening and Reconstruction

Size: 1.24 Mile Cost: \$180 Million

Scope of Work: Reconstructed the Shoemaker Ave. Bridge, the Rosecrans Ave. Bridge, the Bloomfield Avenue Bridge, and the Silverbow Ave. pedestrian overcrossing. Widened one mile of I-5 from Shoemaker Ave. to Silverbow Ave. Added one High Occupancy Vehicle (HOV) lane, or carpool lane, and one general purpose lane in each direction. Widened the freeway shoulders, built concrete median barriers, reconfigured the on and off ramps, and realigned Firestone Boulevard. This project also includes the construction of an Austin Vault, which filters freeway drainage and reduces pollutants.

- Key Features
  - enhance safety
  - add traffic lanes
  - encourage ride sharing through new HOV lanes
  - decrease surface street traffic
  - help improve air quality

Owner Information:

Phone #: (916) 654-5266



## I-5 / Genesee Avenue Interchange 2015-2018

Type: Highway Bridge Widening and Reconstruction

Size: 1.5 Mile

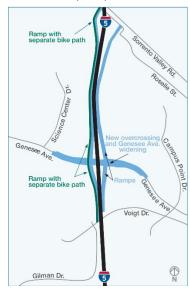
Cost: \$117.4 million

Scope of Work: The Interstate 5 (I-5)/Genesee Avenue Interchange Project will replace the existing six lane Genesee Avenue overpass with a ten-lane bridge that will accommodate current and future traffic demands in the job intensive community Key Features

- Environmental clearance (CEQA/NEPA) which minimizes the impact to sensitive resources
- 2019 Project of the Year, ASCE San Diego
- 2019 Outstanding Project for Structural Engineering, ASCE San Diego
- 2019 Interchange Project of the Year, California Transportation Foundation (CTF)

#### Owner Information:

Phone #: (916) 654-5266



## Alondra Blvd. Project 2012-2015

Type: Highway Bridge Widening and Reconstruction

Size: 0.43 Mile Cost: \$110 million

Scope of Work: Reconstructed the Alondra Blvd. Bridge overcrossing. The new bridge is longer and widened from four to six lanes, with three lanes in each direction. Widened nearly one mile of I-5 from North Fork Coyote Creek to Marquardt Avenue. Added one High Occupancy Vehicle (HOV) or carpool lane and one general purpose lane in each direction; widened shoulders; and constructed a concrete median barrier. Key Features

enhance safety

- add traffic lanes
- encourage ride sharing through new HOV lanes
- decrease surface street traffic
- help improve air quality

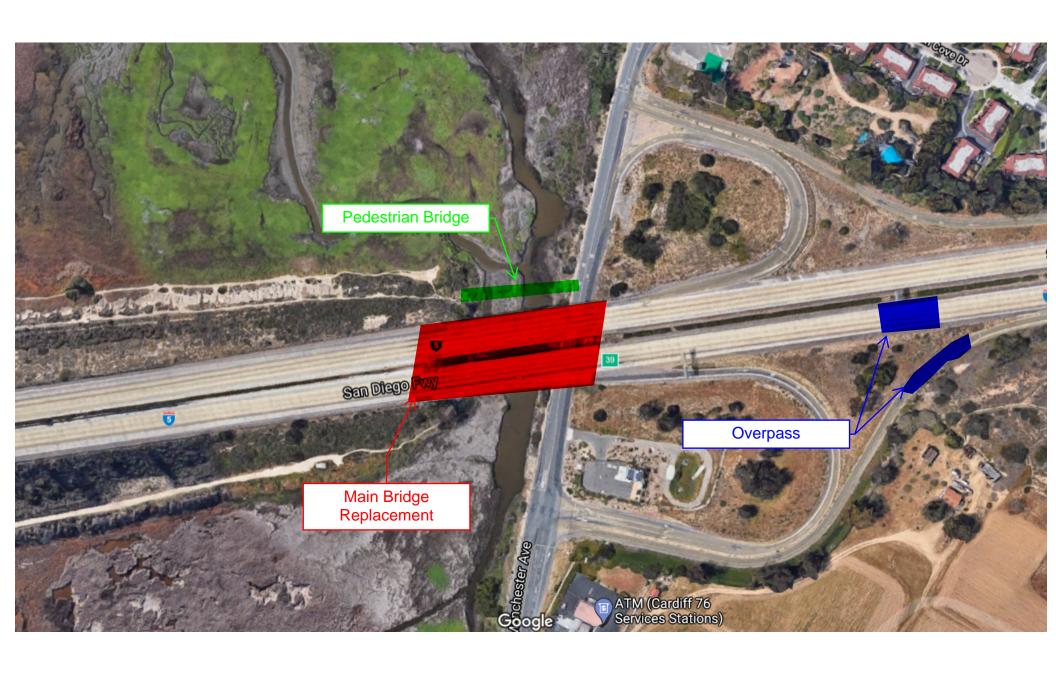
Owner Information:

Phone #: (916) 654-5266

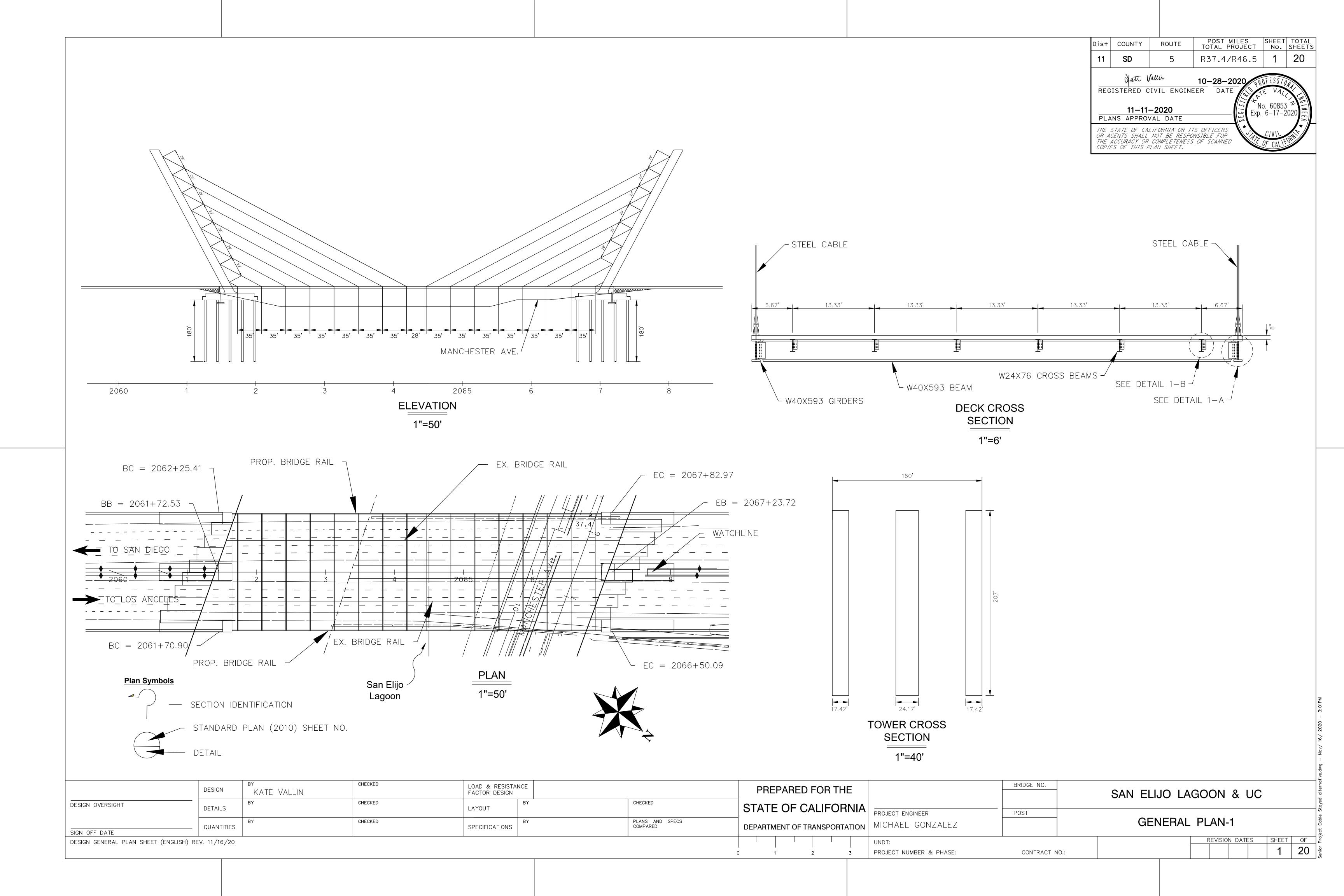
### **Alondra Boulevard Bridge**

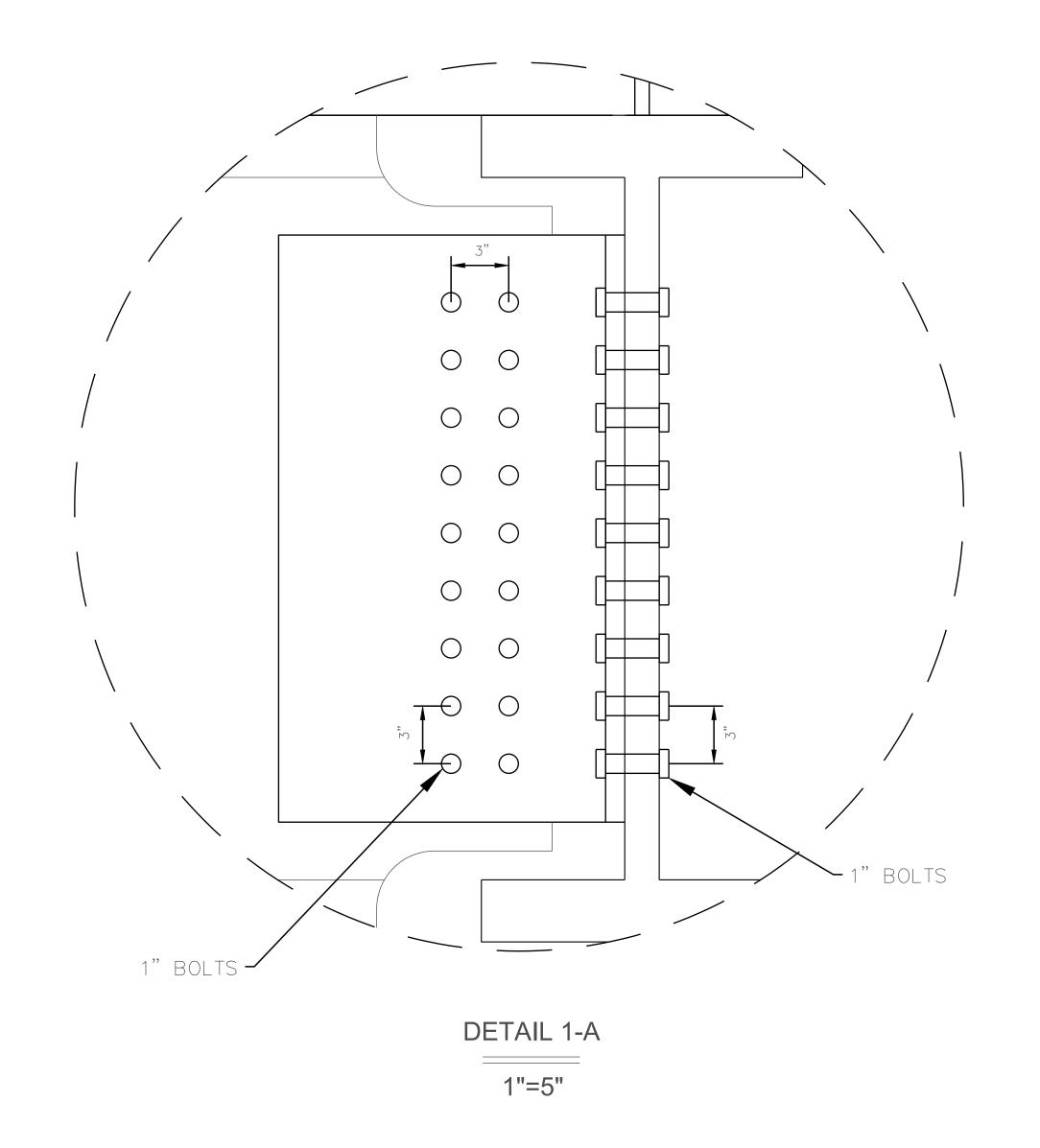


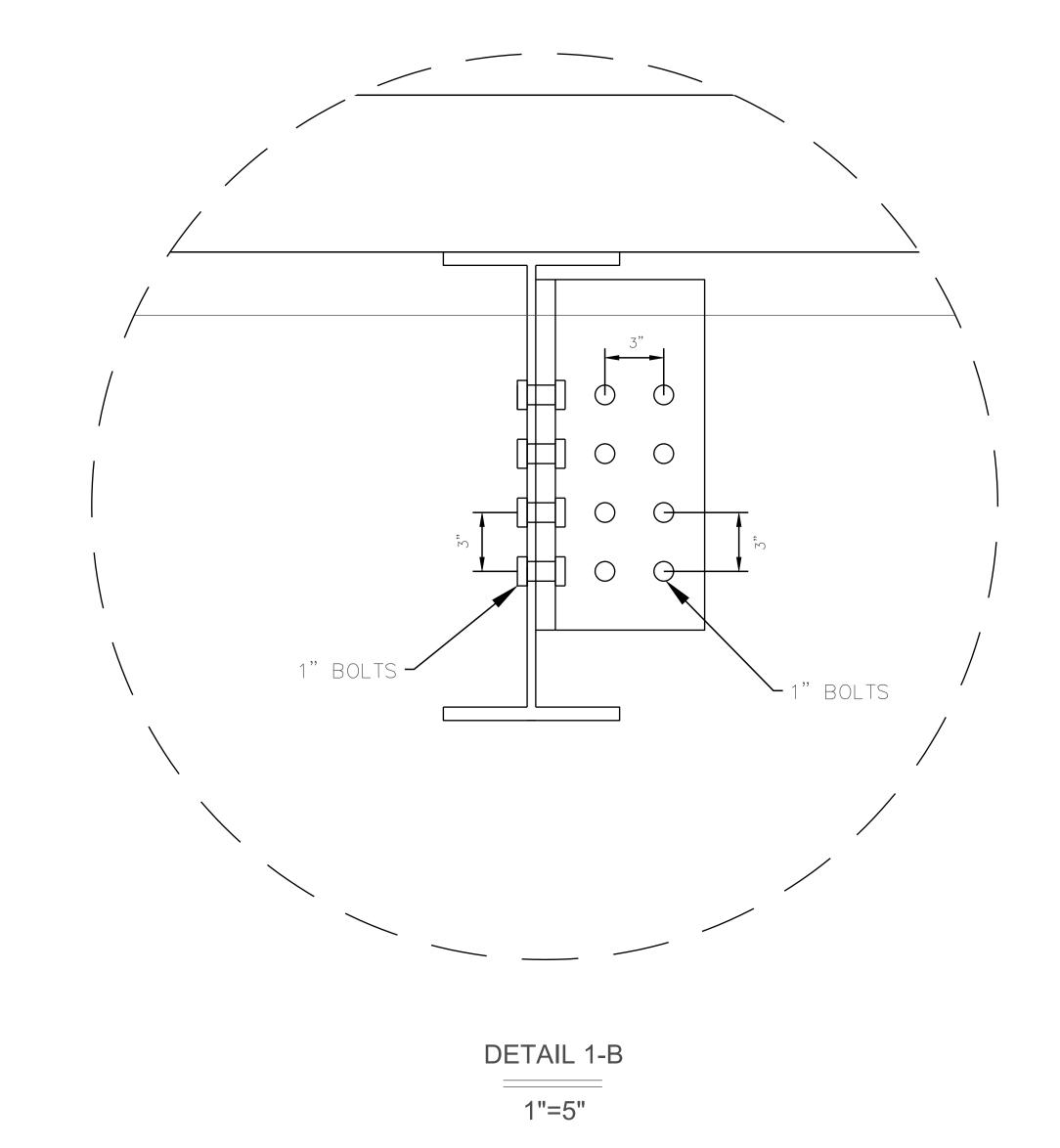
# 10. Site Plan



# 11. StructuralPlans







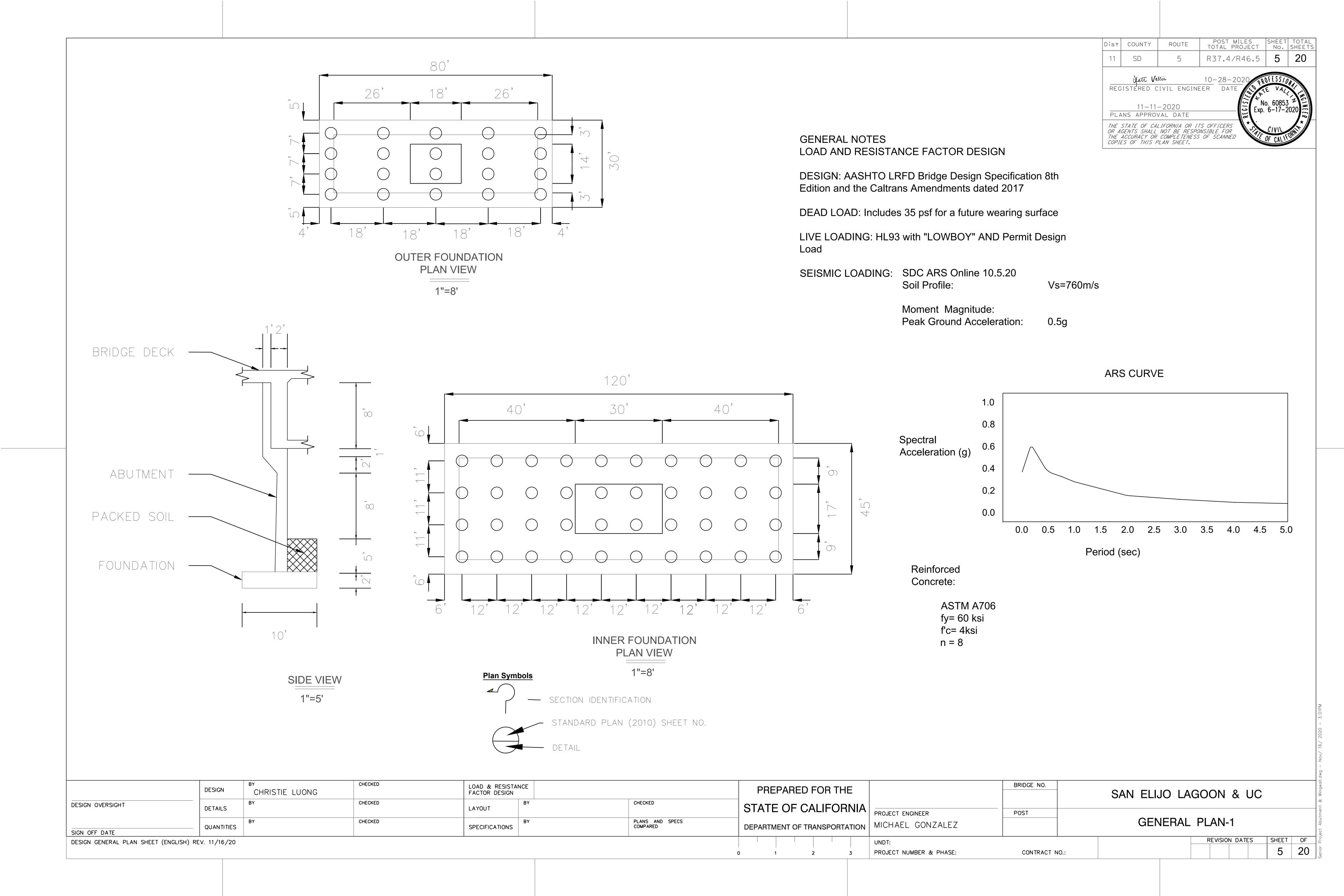
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PLANS APPROVAL DATE

THE STATE OF CALIFORNIA OR ITS OFFICERS
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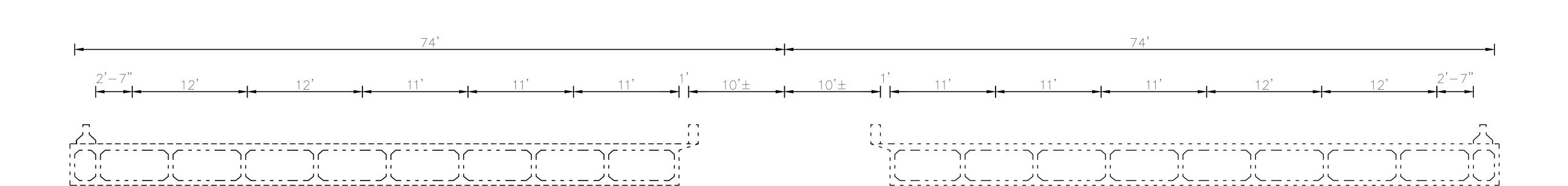
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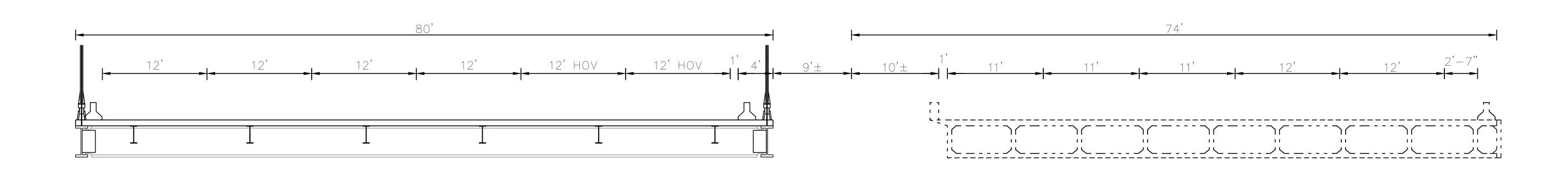
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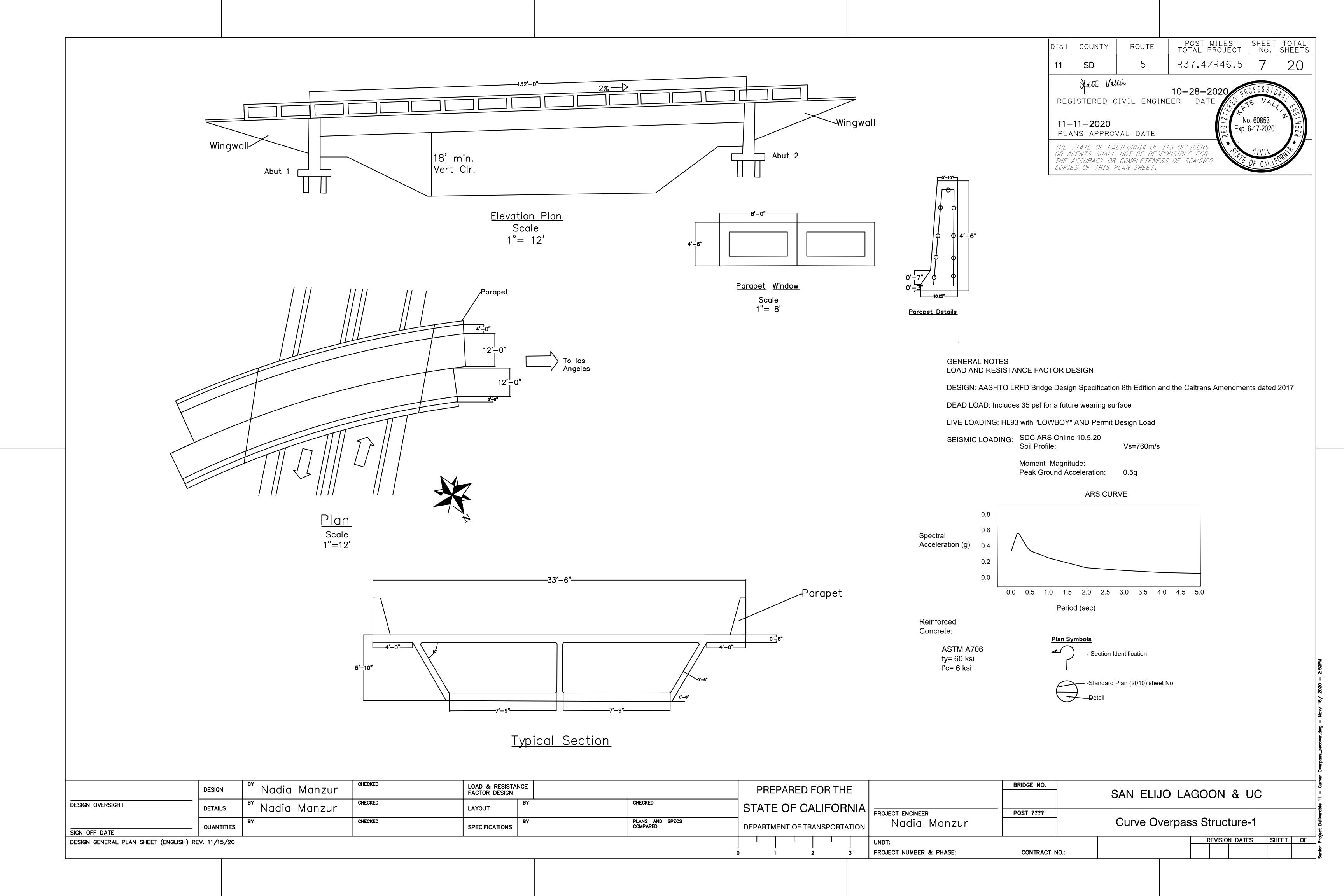
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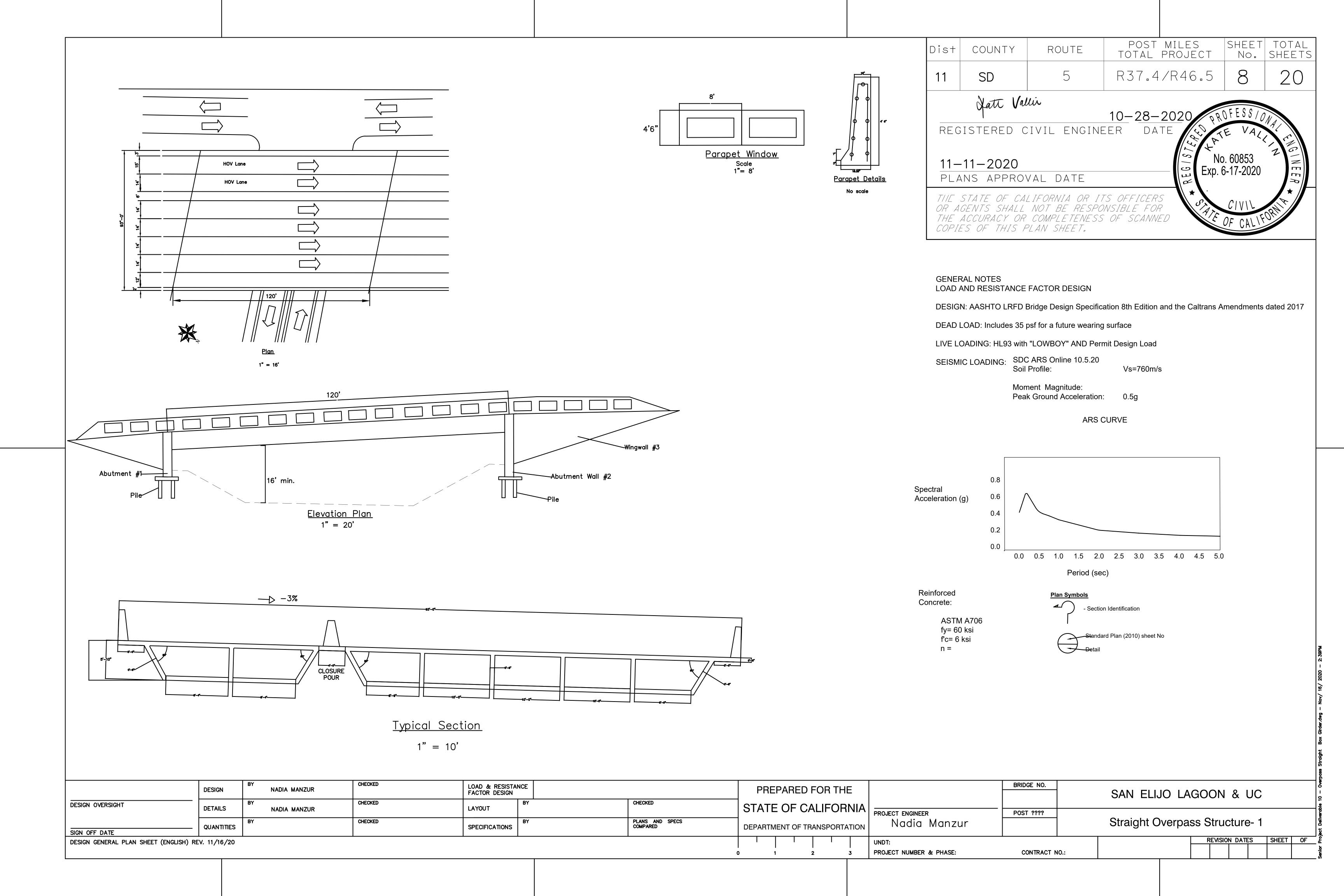
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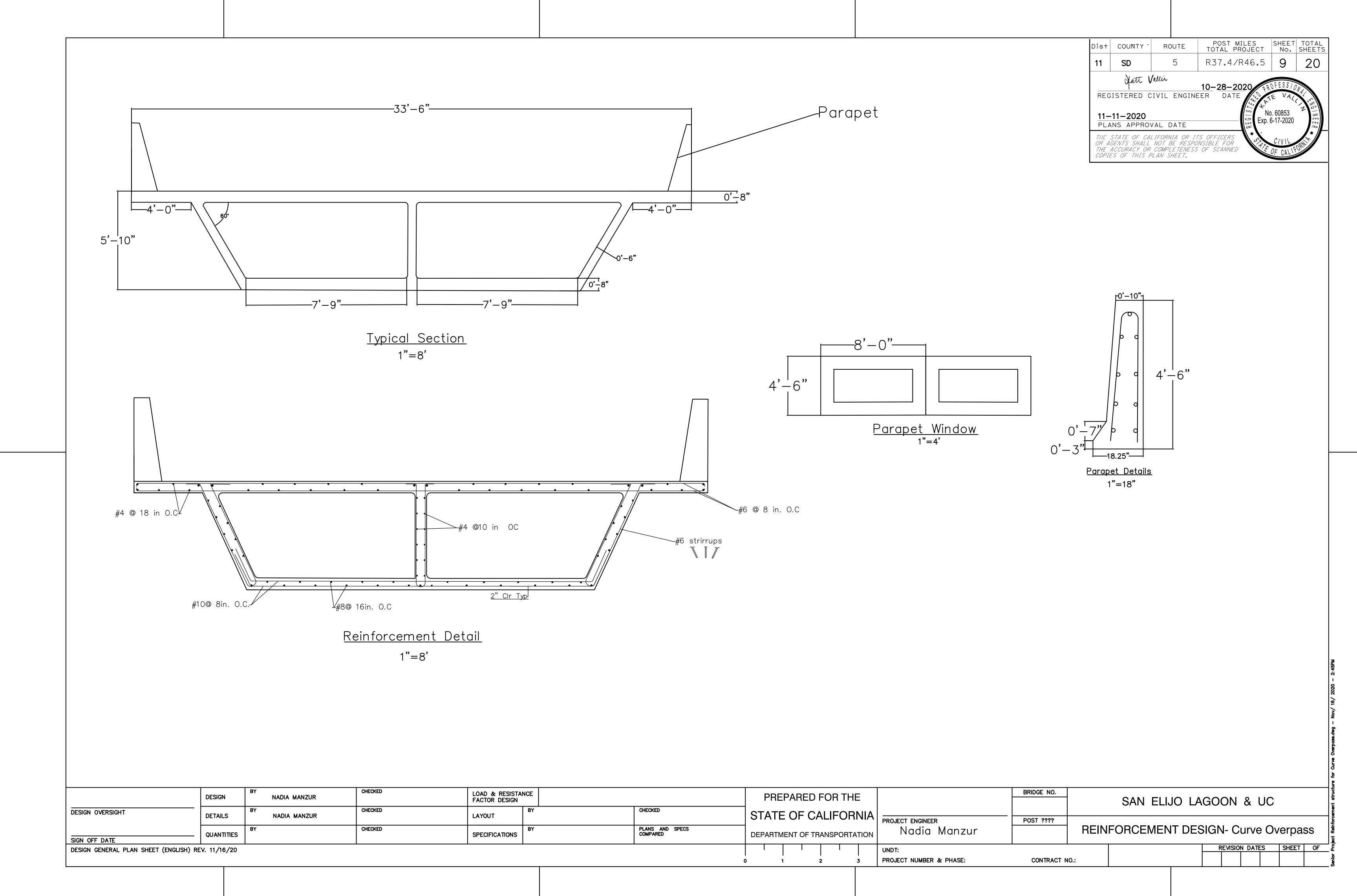
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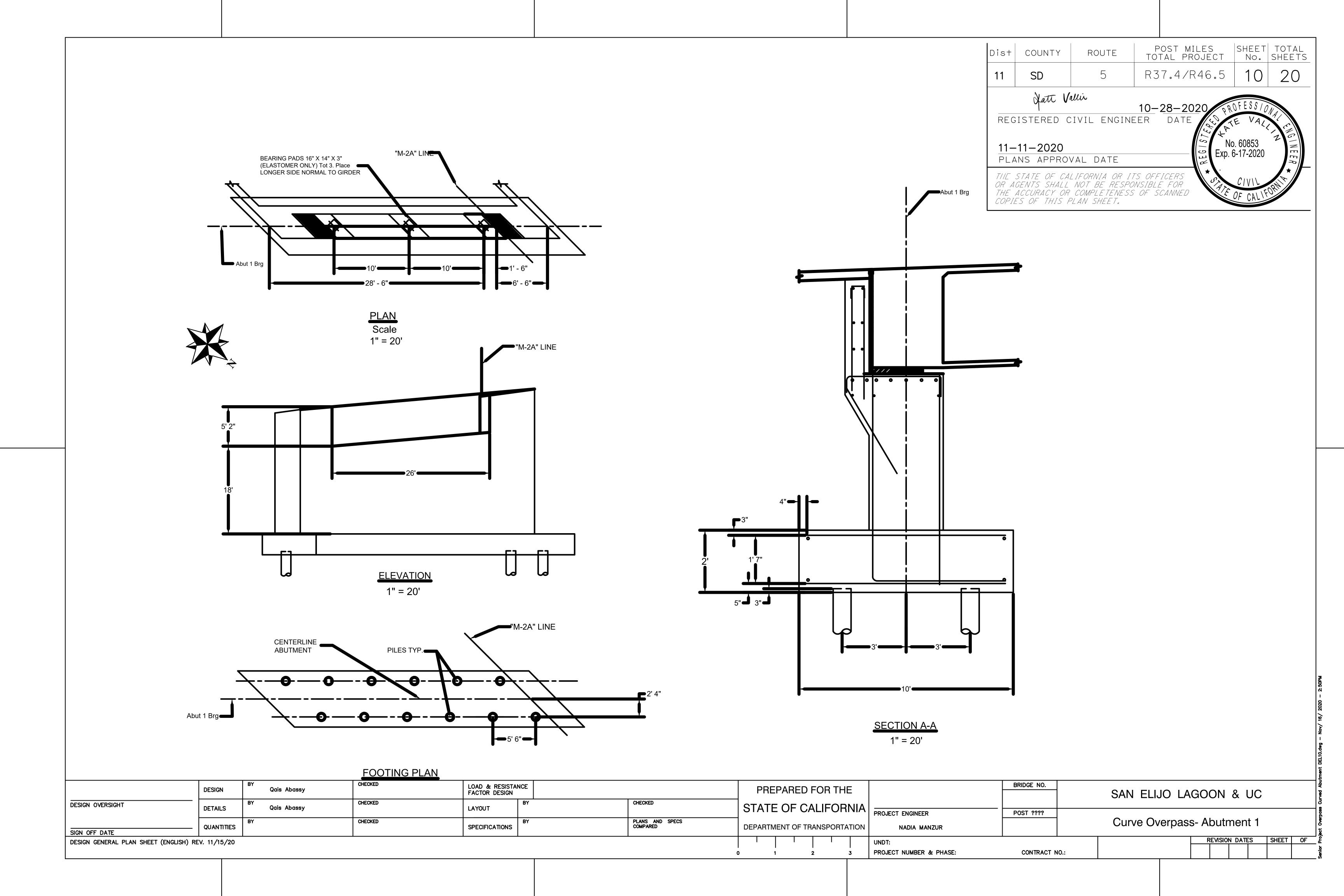
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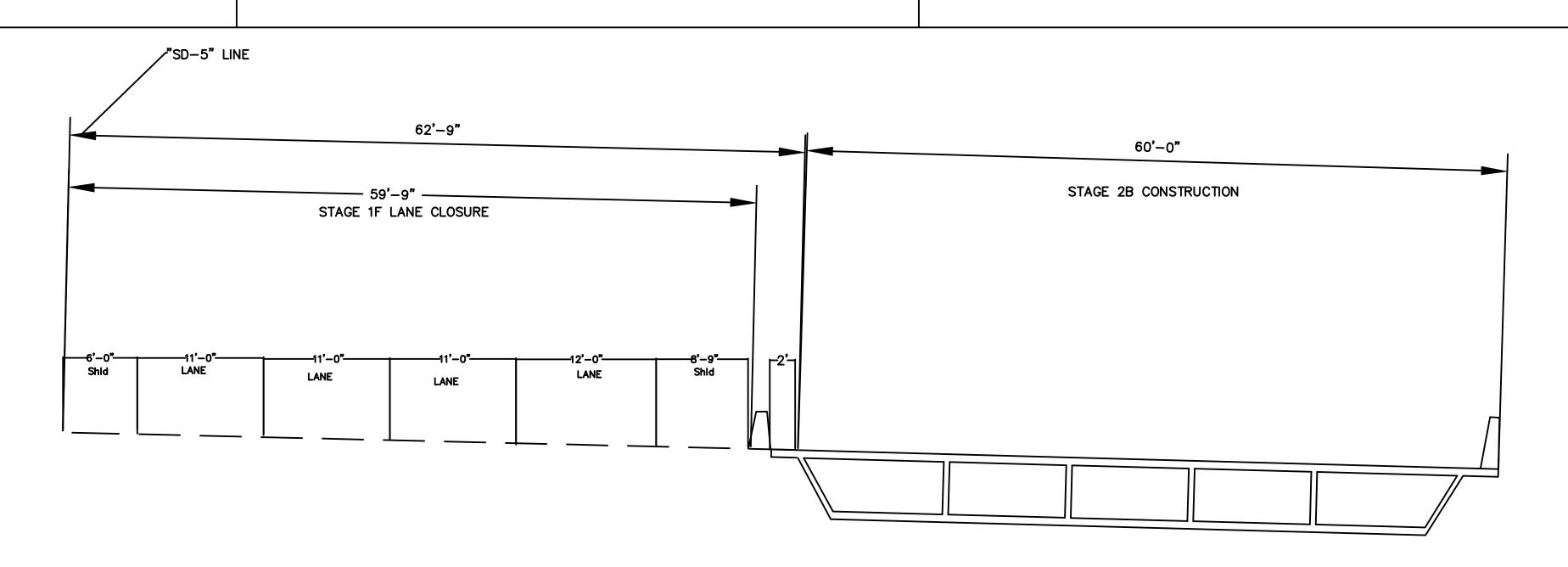
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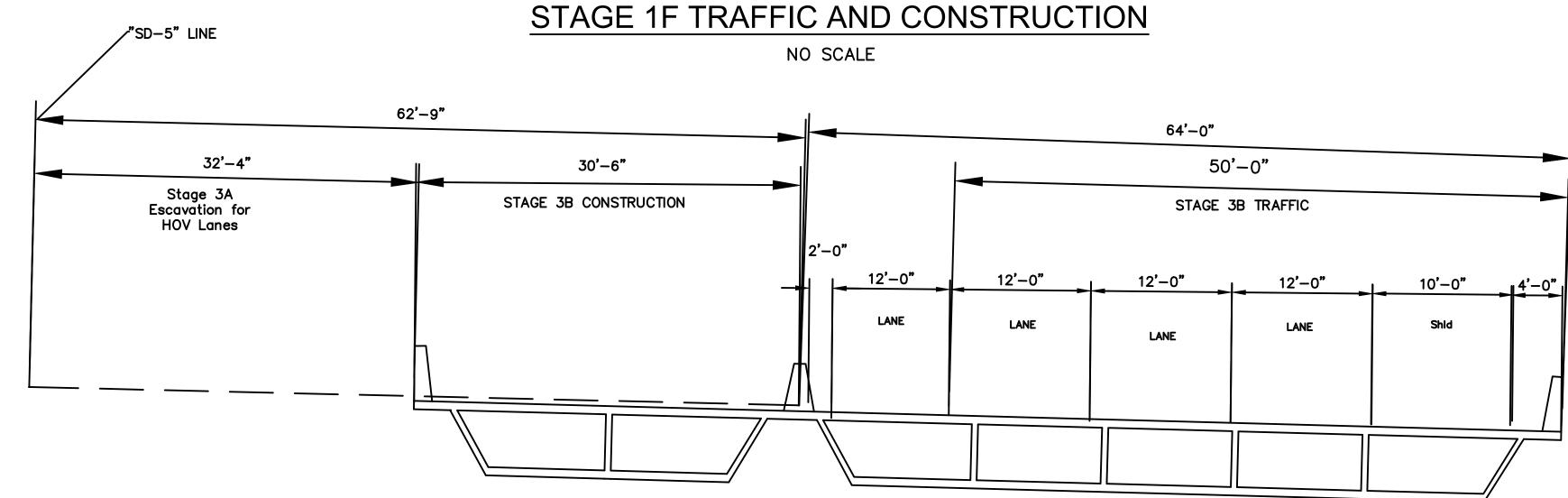






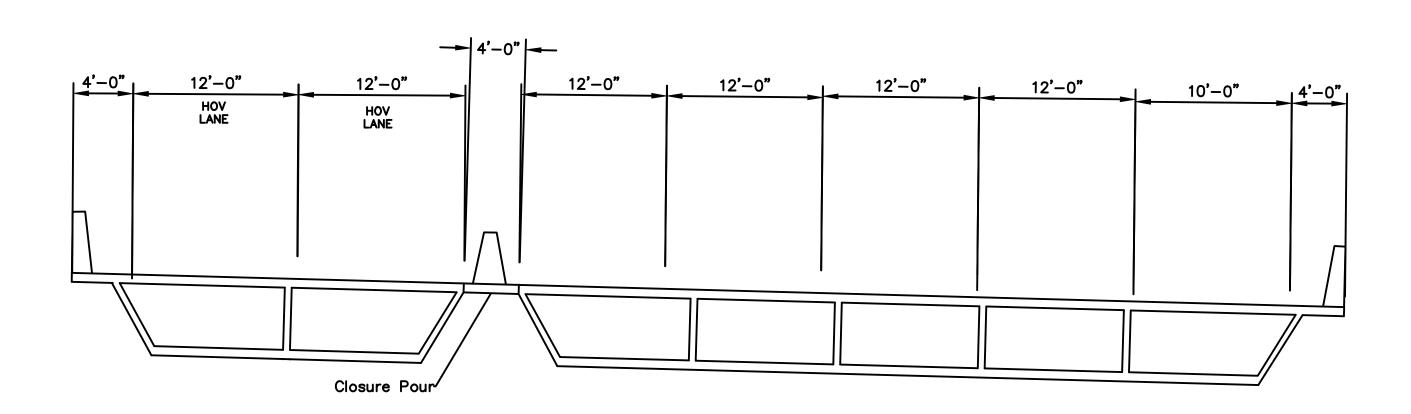






## STAGE 2A TRAFFIC AND CONSTRUCTION

NO SCALE



## STAGE 3B TRAFFIC AND CONSTRUCTION

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**GENERAL NOTES** 

LOAD AND RESISTANCE FACTOR DESIGN

DESIGN: AASHTO LRFD Bridge Design Specification 8th Edition and the Caltrans Amendments dated 2

DEAD LOAD: Includes 35 psf for a future wearing surface

LIVE LOADING: HL93 with "LOWBOY" AND Permit Design Load

SEISMIC LOADING: SDC ARS Online 10.5.20

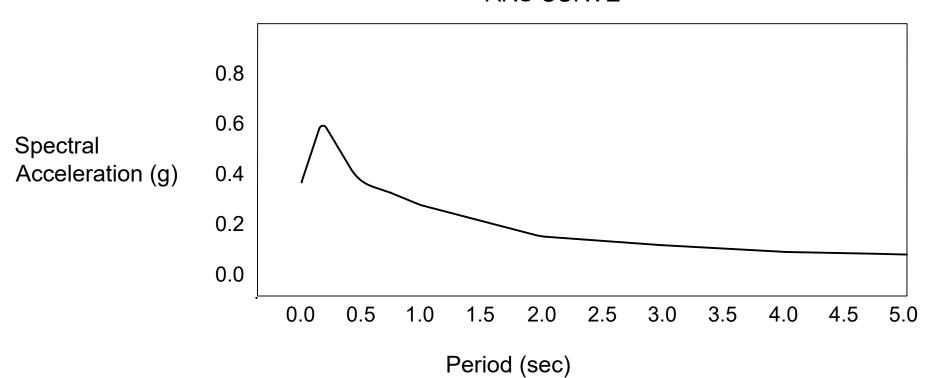
Soil Profile:

Vs=760m/s

Moment Magnitude:

Peak Ground Acceleration: 0.5g

ARS CURVE



Reinforced Concrete:

ASTM A706 fy= 60 ksi f'c= 6ksi - Section Identification

-Standard Plan (2010) sheet No

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GENERAL NOTES

ASTM A706

fy= 60 ksi

f'c= 4ksi

n = 8

LOAD AND RESISTANCE FACTOR DESIGN

DESIGN: AASHTO LRFD Bridge Design Specification 8th Edition and the Caltrans Amendments dated 2017

DEAD LOAD: Includes 35 psf for a future wearing surface

LIVE LOADING: HL93 with "LOWBOY" AND Permit Design Load

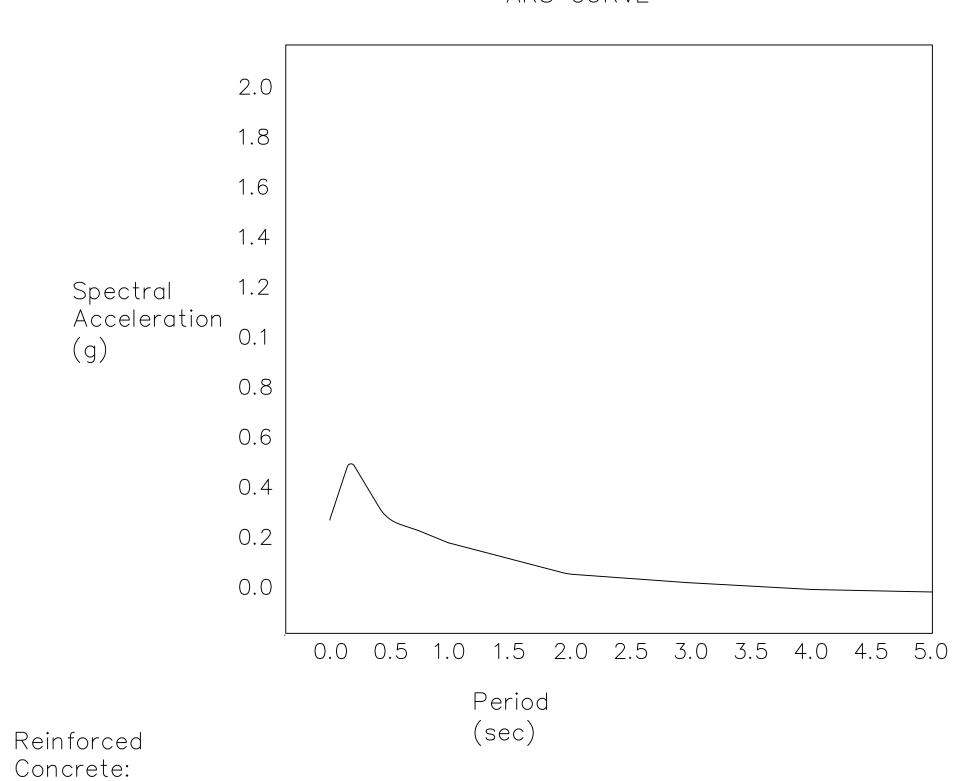
SEISMIC LOADING: SDC ARS Online 10.5.20 Soil Profile:

Vs=760m/s

Moment Magnitude:

Peak Ground Acceleration: 0.5g

## ARS CURVE

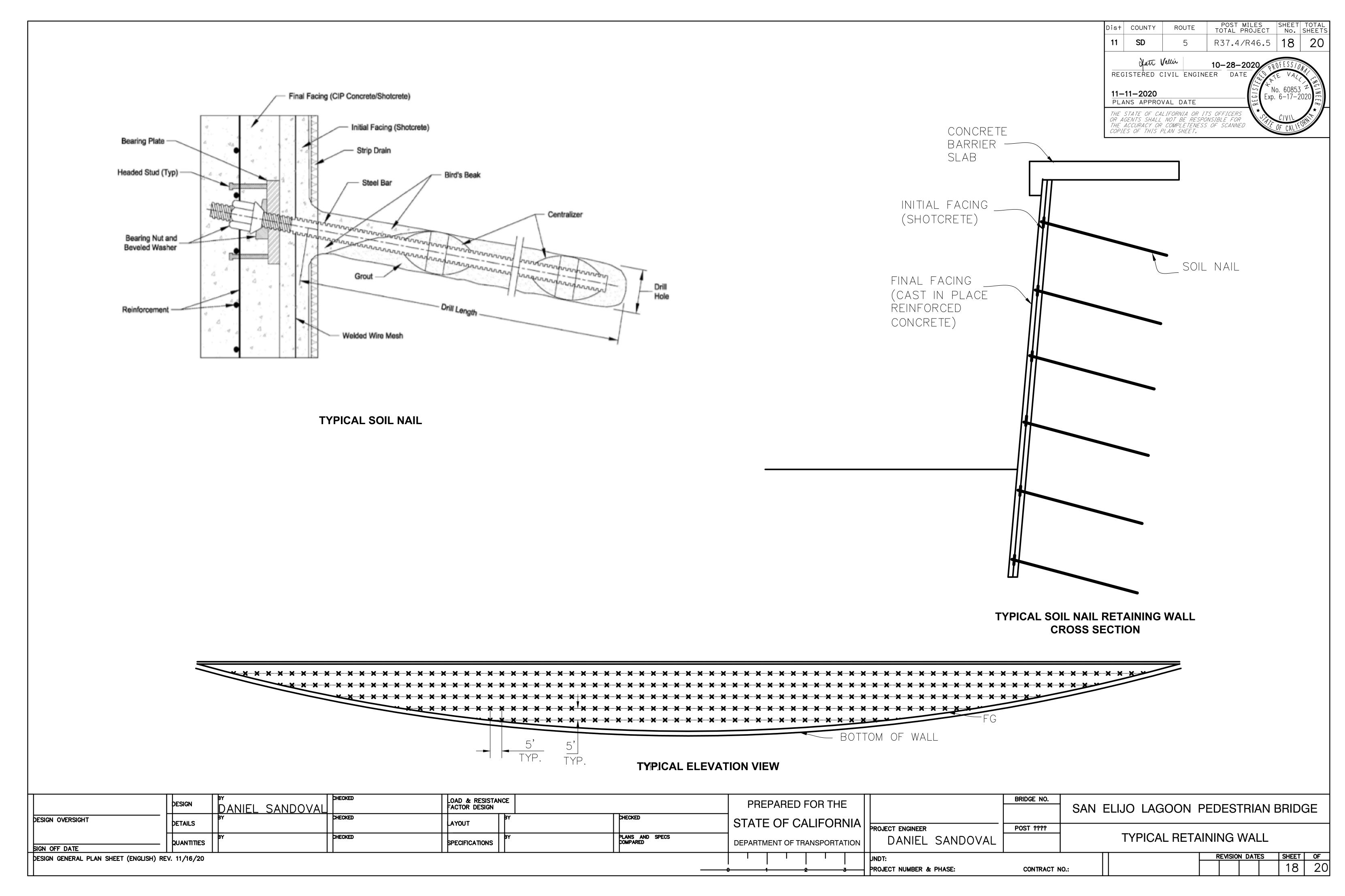


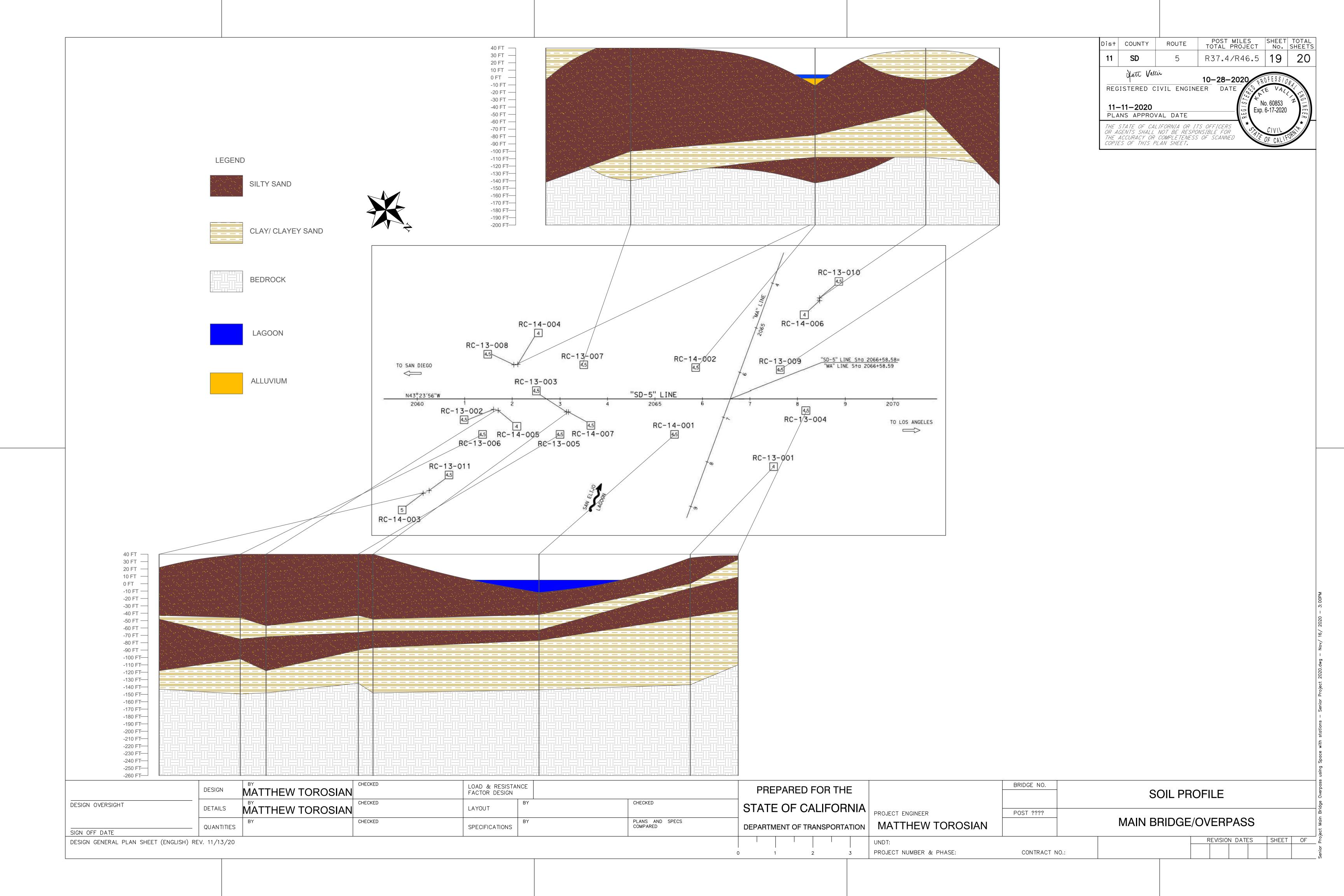
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COPIES OF THIS PLAN SHEET. 14'+4" 10'-0"Min CLEARANCE ENVELOPE 4"NW CONCRETE DECK W21X248
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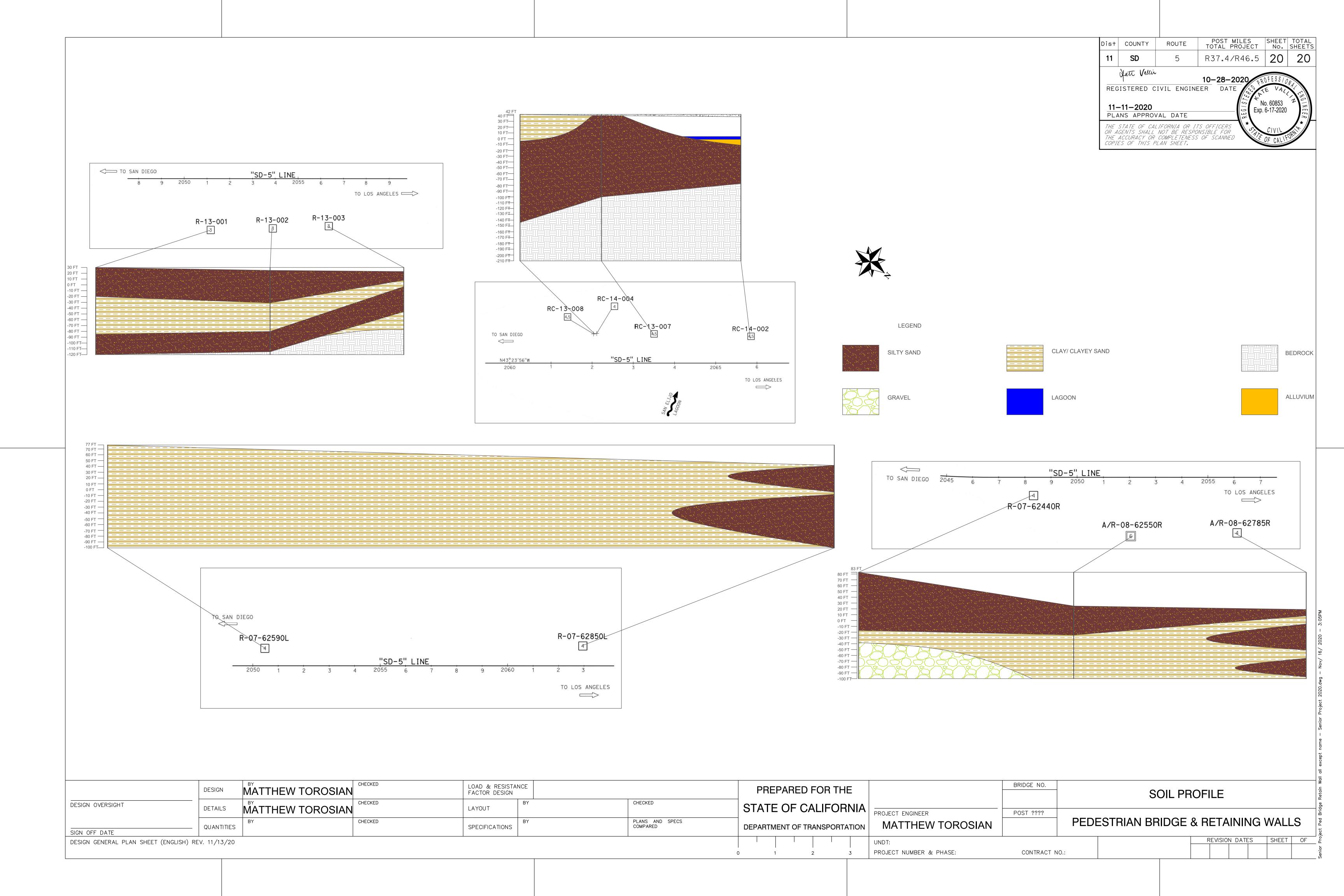
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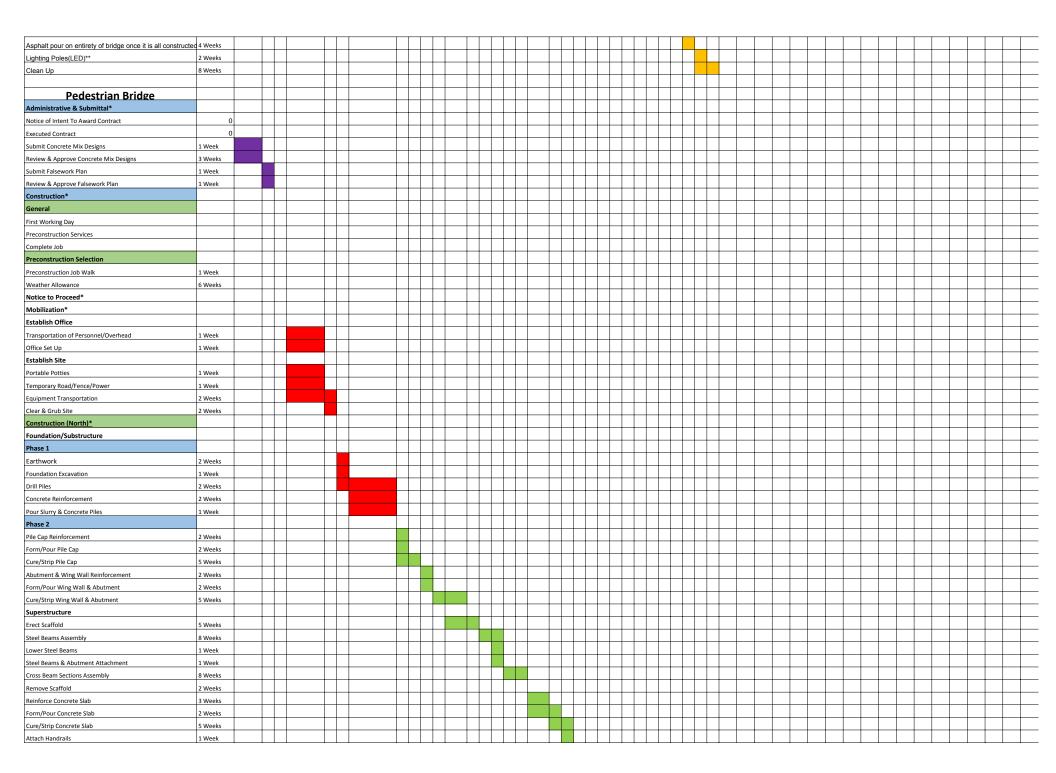


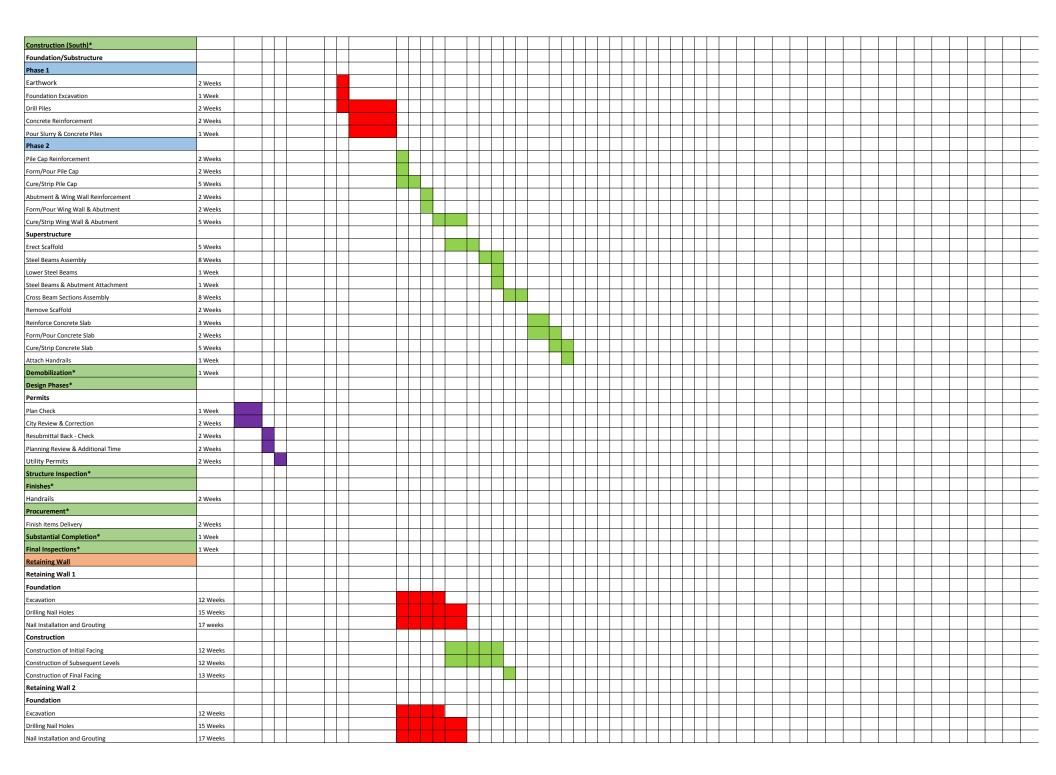
13. Project Schedule, Construction Site Phasing, and Logistics Plan

Admin	Phase 1	Phase 3	
Phase	Phase 2	Phase 4	

#### **BB** Incorporated Schedule

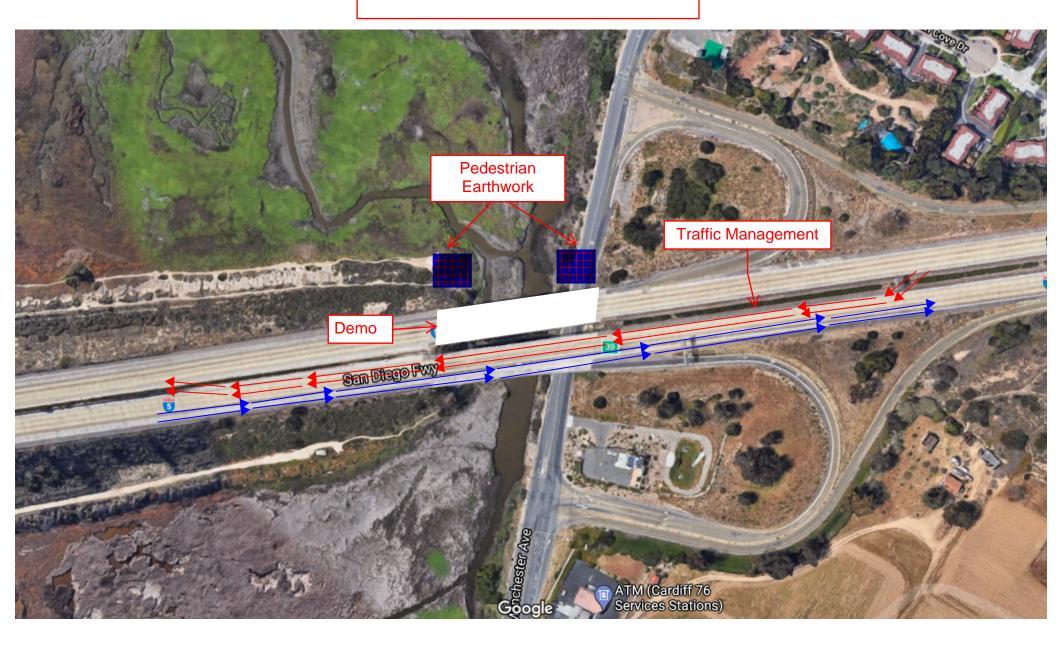
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Preconstruction Activities																										T																		
Prepare Construction Drawing Submittals	4 Weeks																																											
Final Construction Drawing Submittals	4 Weeks																																											
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Prepare Construction Drawing Submittals	8 Weeks																							T																				
Final Construction Drawing Submittals	8 Weeks																																											
Provide Project Specifications																																												
Prepare Technical Specifications (30%, 65%)	20 Weeks				30%						65%																																	
Update Project specifications (95%, 100%)	12 Weeks			П											1	.00%								T																				
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Abutment Pouring	32 Weeks									+		+		+				+			+	Н		+		+								+							$\vdash$		$\vdash$	_
	32 Weeks			Н						+		+						+		+		Н		+		+															$\vdash$		$\vdash$	
Tower Foundation (Pile Caps) layout Tower Foundation (Pile Caps) Pouring	32 Weeks			H						+		+										H														-					$\vdash$		$\vdash$	
	32 Weeks			H								+		+										+		+															$\vdash$		H	
Superstructure	16 Weeks		+	H				+	+				$\vdash$			-		+								+							+	-	+	+	+				$\vdash$		$\vdash$	_
Scaffold Erection	36 Weeks		+					+	+					+		$\rightarrow$		+		-			_			+	+						+		+	+	+	_			$\vdash$		$\vdash$	_
Shore Forms Steel Formwork Leveut		+	+	$\vdash$		+		+	+	+					$\vdash$	$\dashv$	+	+	$\vdash$					+	+	+	+					+	+	-	+	+					$\vdash$		$\vdash$	_
Steel Formwork Layout	36 Weeks	+	+	H		+		+	$\vdash$	+					$\vdash$	$\dashv$	+	+	$\vdash$					+	+	+	1					+	+	+	+	+							$\vdash$	
Welding Metals		1	+	$\vdash$		+		+	$\forall$	+					$\vdash$	$\dashv$	+	+	$\vdash$					+		+	+		_	-		+	+	+	+	+	+				$\vdash$		$\vdash$	
28' Reinforced Steel Casing Layout	36 Weeks	+	+	$\vdash$		+		+	+	+						-	-	+	$\vdash$							+	+			1		+	+	+	+	+	+	$\vdash$		_	$\vdash$		$\vdash$	
28' Concrete Tower Pouring	36 Weeks	+	+	$\vdash$		+		+	+	+							-	+	$\vdash$	_						+	+			1		+	+	+	+	+	+	$\vdash$			$\vdash$		$\vdash$	
Cable system assembly	12 Weeks	+	+	$\vdash$		+		+	$\vdash$	+	+	+					+	+	$\vdash$	+				П	$\vdash$	+	-			-		+	+	-	+	+	+	_			$\vdash$		$\vdash$	
Layout and Construct 2 Outside Griders	32 Weeks			H		+		+	H	+	+	+				-	+	+	$\vdash$	+	F		+		$\vdash$	+	-	-				-	+	+	+	+					$\vdash$		$\vdash$	
Deck Assembly - 35' sections	32 Weeks								$\perp$	$\perp$																	1	1						_	_		1				Щ.		ш	





Construction	
	<del></del>
Construction of Initial Facing 12 Weeks	
Construction of Subsequent Levels 12 Weeks	
Construction of Final Facing 13 Weeks	
Overpass Ove	
Admin Phase Service Se	
Pre Admin Work 9 Weeks	
Design Phase	
Complete Design 9 Weeks	
Contract Execution*	
Award Project 1 Week	
Weather Allowance 5 Weeks	
Notice to Proceed*	
Mobilization*	
Install Construction Area Yard 1 Week	
Install Construction Area Signs 1 Week	
Install BMP's 1 Week	
Temporary Stripe 1 Day	
Install k-Rail 1 Day	
Clear & Grub Site 2 Weeks	
Start Construction	
Straight Overpass	
Foundation	
Earthwork 3 Weeks	
Structure Excavation 4 Weeks	
Drill Pipes 4 Weeks	
Form and Pour Piles 4 Weeks	
Form and Pour Bridge Foundation 2 Weeks	
Form and Pour Abutments 2 Weeks	
Form and Pour Wing Walls 2 Weeks	
Install Falsework 4 Weeks	
Install Box Girders 1 Week	
Install Deck Form with Falsework 2 Weeks	
Form and Pour Diaphragm 1 Week	
Form and Pour Barrier 1 Week	
Form and Pour Deck 1 Week	
Form and Pour HOV Lanes 2 Weeks	
Install Railing 1 Day	
Stripe 1 Day	
Remove k-Rail 1 Day	
Start Construction	
Curved Overpass	
Foundation	
Earthwork - Clear and Grub - Ground Prep 6 Weeks	
Structure Excavation 5 Weeks	
Drill Pipes 5 Weeks	
Form and Pour Piles 4 Weeks	
Form and Pour Bridge Foundation 4 Weeks	
Form and Pour Abutments 2 Weeks	
Form and Pour Wing Walls 2 Weeks	
Install Falsework 5 Weeks	
Install Box Girders 2 Weeks	
Install Deck Form with Falsework 2 Weeks	
Form and Pour Diaphragm 1 Week	

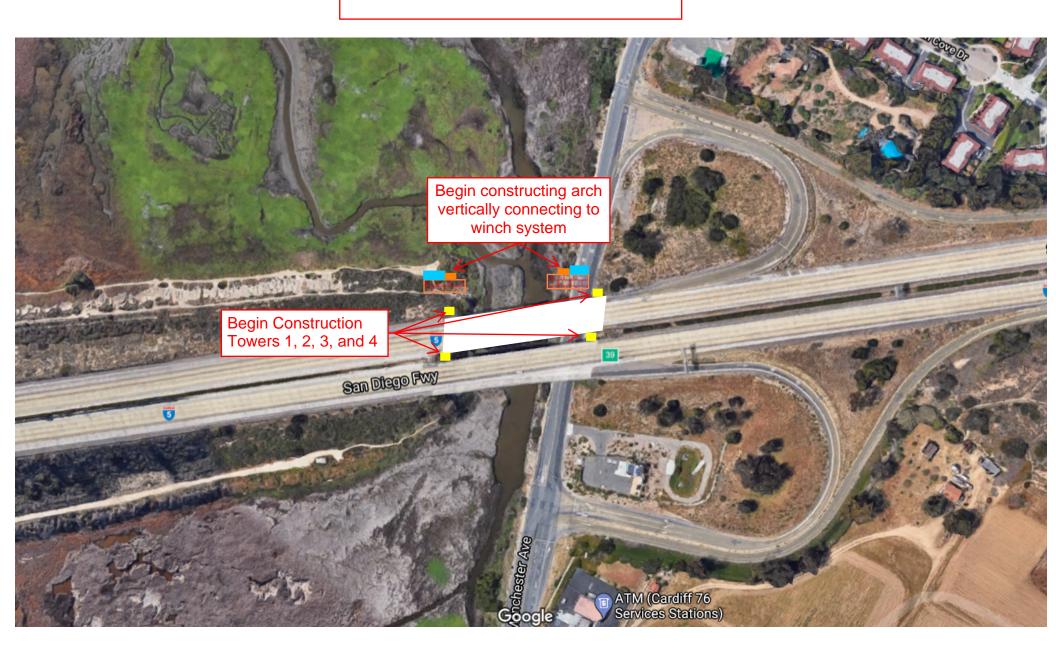
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Form and Pour Barrier	2 Weeks	Ш		$\perp$	Ш	┙	Ш	Ш		Ш	┸		Ш		Ш	Ш				$\perp$					ш		
Form and Pour Deck	1 Week																										
Form and Pour Approach and Detour Slabs	2 Weeks																										
Install Railing	1 Day																										
Stripe	1 Day																										
Remove k-Rail	1 Day																										
Punch List	4 Weeks							П																	$\Box$		
Demobilization	1 Week							П					П														
Design Phases*								П					П			П									П		
Permits								П																	П		
Plan Check	1 Week				П			П					П												$\Box$		
City Review & Correction	2 Weeks							П																	$\Box$		
Resubmittal Back - Check	2 Weeks							П					П														
Planning Review & Additional Time	2 Weeks							П					П												П		
Utility Permits	2 Weeks						П	П																			
Structure Inspection*						T	П	П					П												$\Box$		
Finishes*																											











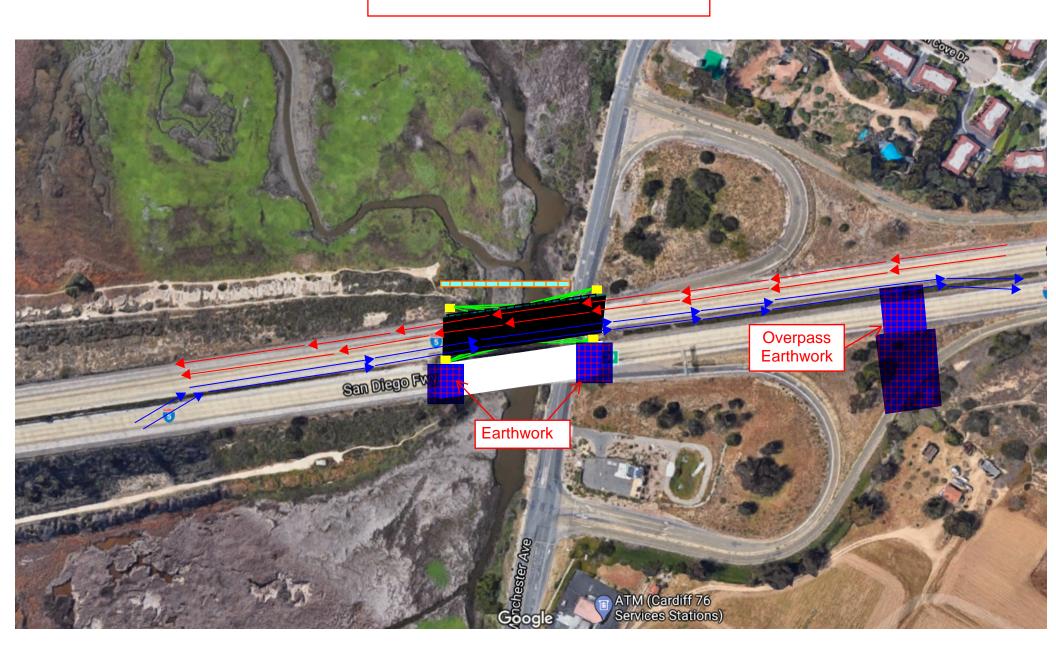


















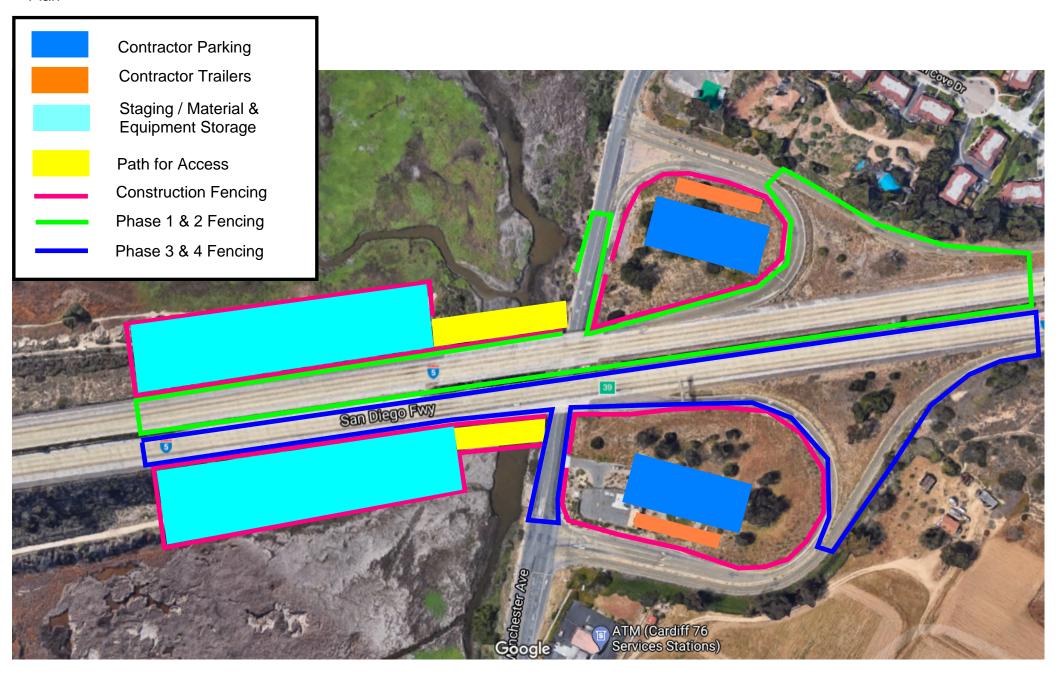








BB Incorporated Site Logistics Plan



# 14. Technical Report

## **Design Report**

#### **General Description**

The innovative design for the Main Bridge Replacement was conjured up in the hopes to build an efficient and sustainable structure. The idea for a cable-stayed bridge was devised in order to keep the San Elijo Lagoon untouched in efforts to preserve the current ecosystem.

#### **Materials**

The bridge design is made possible through the use of inclined pylons. They consist of steel caissons that are erected and filled with concrete. For the bridge deck, it consists of steel W shape members that are connected with steel bolts. The slab that sits above the deck is made of concrete and asphalt pavement.

### **Description of Structural Members**

The structural elements of the main bridge replacement are made up of a series of parts that work together to maintain an efficient design. Without one of these elements the design would not be possible. The deck of this bridge is made up of steel W shape members which will be held up by steel cables connected to 3 pylons on each side of the bridge. These cables transfer the dead and live load from the deck to the towers. To avoid installing anchors, the towers were tilted to 62 degrees from horizontal to utilize the towers' self counteract the weight experienced on the deck. These towers then take all the compression and tension forces and transfer then down into the foundations. The foundations then transfer the loads through the piles that are drilled into bedrock.

#### **Loads Analysis**

Utilizing the loads described in Table A, and specifications in the ACI 318 Building Code, the following elements were determined to be the most efficient for the loads being experienced and and the overall cable stayed design. A model of this bridge was created using SAP which computed the maximum positive and negative moments of the bridge and those values were utilized to optimize the design. Images of the SAP model can be seen in Figures 1-4.

Load Type	Description	Load psf
Dead Load	Self-weight of the structures permanent elements Unit weight of concrete= 150 pcf Pavement weight = 40 pcf Barriers (line load) = 250 plf	152.92
Design Lane Load	Live load produced by a lane on the bridge superstructure due to vehicular traffic = 0.64 k/ft	53.33

Table A: Loads applied in preliminary design calculations of deck

## **Preliminary Design**

In designing the deck for the cable-stayed bridge, it was determined that the thickness of the deck to be 8". This parameter allowed the calculation of the box girder's flange spacing using ACI Table 7.3.1.1. The outer flanges of the box girder were determined to be 6'8" from the edge of the slab edges utilizing  $h_{min} = \frac{l}{10}$  for a one way slab with a cantilever end. The middle four flanges were then determined to be spaced at 13'4" on center through multiplying 6'8" by two. The use of 13'4" was checked with  $h_{min} = \frac{l}{28}$ , or the minimum slab thickness equation for a one way slab with continuous ends. l was found to be 18.67'. Thus, as 13'4" is less than 18.67' then the middle four flanges spaced at 13'4" on center is acceptable. A series of checks were executed based on ACI compliance for our slab design. These checks include minimum height of section, minimum width of section, minimum steel reinforcement, Ductility check and DCR check. As can be seen in Tables C and D, the slab passes all checks for both positive bending and negative bending.

The bridge replacement team has incorporated a series of Cable- Stayed ideas and arrived at an innovative yet efficient design for this project. The design of the pylons is inspired by the Alamillo Bridge in Seville, Spain whose idea was devised by Santiago Cavaltrava. The bridge features three towers that will each support the loads experienced on the deck and carried by the steel cables. These towers are tilted 62 degrees from horizontal and are made of steel caissons and concrete to counterbalance the weight from the deck. The cables are pin connected to the tower and anchored to the deck each at 28 degrees from horizontal. The total height of the pylons was determined to be 207 feet as was calculated to be the most efficient height for this preliminary design.

#### **Preliminary Deck Design Summary Table**

## **Design Factors**

f'c: 5000 psi Es : 29000ksi

#### Dead Load:

Concrete Unit weight: 150 pcf Pavement Unit Weight: 140 pcf

Barrier: 250 pcf

#### Live Load:

HL-93 & Permit Design Load (point loads): 1.44 k/ft

#### Element

#### **Pavement**

Thickness: 4 in. Acceptable Design

#### Slab

Thickness: 8 in.
Acceptable Design:

## **Girder -** W40X593

Depth: 43 in.

Flange Width: 16.7 in. Flange Thickness: 3.23 in. Web Thickness: 1.79 in.

#### Cross-Beam - W24X76

Depth: 23.9

Flange Width: 8.99 Flange Thickness: 0.68 Web Thickness: 0.44

## Beam- W40X593 Depth: 43 in.

Flange Width: 16.7 in.
Flange Thickness: 3.23 in.
Web Thickness: 1.79 in.

## **Design of Abutment**

The process of the abutment calculations were made up of a series of iterations because checks needed to be done in order to get a reasonable factor of safety (FS) according to CalTrans standards. The overturning moment calculation resulted with a factor of safety of 2.74 which is greater than 1.5 the minimum, therefore the abutment dimensions are good to resist overturning. For sliding calculations, we found that the factor of safety resulted in 2.64 which is greater than 1.5, therefore, it met safety factory requirements. However, the bearing capacity was calculated and it resulted in a safety factor of 2.639 which is less than the safety factor of 3 needed in the bearing capacity calculation, therefore showing that piles are needed for this bridge. Each variable that goes into the bearing capacity was taken from the soil report that was tabulated in the soil parameters taken from the RFP.

This bridge will have a total of 4 abutments with two on each end of the bridge. Per the design calcs shown in table B, the bridge load that will be acting on one abutment is 339.64k. This load along with the self weight of the abutment, the weight of the pylons, and the weight of the foundations will be acting on the piles. Doing these calculations results in about 19.378 piles which tells us that we need 20 piles for each abutment. However, the weight of 339.64k acting on the abutment is lower than the bearing capacity of one pile which is 1660.74k. Therefore, taking this into consideration, the abutment itself will not have any piles.

#### **Design of Footings**

The design of the footings were determined based on the load of the bridge deck and the load of each pylon that needs to be supported. The design for the footing had to be large enough to resist those loads. There are three footings in which the middle footing is twice as large as the two footings on the sides. This is due to the fact that the middle foundation carries half of one bridge and half of the other. The footing that goes below the middle antenna is 120' by 45' with 40 piles and the two side footings that accommodate the two smaller antennas will have dimensions of 80' by 30' with 20 piles. This totals to 160 piles for the entire bridge. The foundations for the middle pylon will be 20 feet in depth with a step of 10 feet. For the foundations of the two smaller pylons will be half the middle foundation which will be 10 feet in depth with a 5 feet step. The piles will be distributed along all three footings, in which there will be 40 in the middle footing and 20 on the two smaller footings. The piles on the South side of the bridge, going towards San Diego, will have a length of 180 feet in depth and the

piles on the North side of the bridge, going towards Los Angeles, will have a length of 160 feet in depth.

#### **Settlement and Lateral Calculations**

The first step was to obtain the initial soil layer thickness. Then the dimensionless bearing capacity index variable was obtained from the AASHTO federal highway manual. Then the soil unit weight and the length of the abutment was used to compute the values of effective vertical stress and the average change in vertical stress. Once all of the parameters were determined the settlement was calculated. For the lateral loads, the stress on the pile was computed to be 10.98 ksf.

## **Geotechnical Design Calculations**

## **Overturning Calculation on Abutment**

 $\Sigma$  Wi = 30340.5 lb/ft

 $\Sigma$  Mo (Resultant Moment) = 240348.5535 lb-ft/ft

Overturning Moment = 87750 lb-ft/ft

FS = 2.739 > 1.5 **GOOD** 

## **Bearing Capacity**

qu = 23700.61

e = 0.5

qtoe = 8802.32 \*governs

gheel = 4721.73

FS = qu/qtoe = 2.693 < 3 **FAIL** need piles

#### **Sliding Calculation of Abutment**

Kp = 4.2

Pp = 12287.27

D = 6 ft

B = 10ft

FS = 2.46 > 1.5 **GOOD** 

#### **Piles**

Weight on abutment = 339.64 k

Total loads on piles = 41904.k

# of piles needed = 160 piles

D = 4 ft

Ls (length on south side) = 180ft into bedrock

Ln (length on the north side) = 160ft into bedrock

## **Settlement and Lateral Load**

Stress on piles = 10.8 ksf, 10.93 ksf Settlement differential = 0.43 in

Table B: Deck Design Summary Table

ACI (	Complia	nce Checks	(Positiv	ve Bending)
		Geometry	у	777
Top Base	bt	960 ir	n	
Base Web	bw	72 ir	n	
Height	h	75 ir	n	
Effective depth	d	72 ir	n	
Length	L	80 ft	t	
Dead Load	WD	32.05 k	k/ft	
Live Load	WL	13.84 k	k/ft	
		Materials	s	
Compressive Strength	fc	5 ks	si	
Yield Strength	fy	60 ks	si	
Type of Concrete	19000		0001	Normal Weight or Light Weight
7.0.51 71.0.00 7.00		Structural Ana	alysis	
Load Combination 1	Wul	44.87 k/	/ft	1.4WD
Load Combination 2	Wu2	60.604 k	k/ft	1.2WD+1.6WL
Governing Load Combination		60.604 k	/ft	
Moment	Mu	4240.37 k-	-ft	
	ф	0.9		
Nominal Moment	Mn	4711.5222 k-	-ft	Mu/ф
	Z	64.8 ir	n	=0.9*d
	T	872.50412 ki	ip	Mn/Z
Area of Steel Required	As,req	14.5 ir	n^2	
		Checks and Dra	awings	
Select Steel Reinforcement Chec	k			
Number of rebars	n	12		
Rebar cross-sectional area	A	1.56 ir	n^2	
Area of Steel	As	18.72 in	1^2	
Reinforcement Check	98990	PASS	2000	
Minimum Section Width Check				
Minimum width	bmin	36.62 in	1	From Table 5
Section Width Check	er-en-er	PASS		
Minimum Section Height Check				
rebar diameter	db	1.128 in	1	from Table 5
strirrup diameter	ds	0.375 in	1	
minimum clear cover	Cc	1.5 in	1	
Minimum Height	hmin	74.439 in	1	hmin=d+db/2+ds+Cc
Minimum Height Check		PASS		
Minimum Reinforcement Check		-		
Percent of Steel	ρ	0.0036111		As/bd
Minimum Percent of Steel	ρmin	0.0035		from Table A.7
Reinforcement Check	A. C. C. C.	PASS		
Ductility Check		70.50		
Yield Strain of Steel	εcu	0.003 in	ı/in	
Strain of Steel	50000000	A TOTAL CONTROL OF THE		As*fy/0.85*f'c*b
	a	3.6705882 in	10	
	β	0.8		From Table A.7
	С	4.5882353 in		а/β
	ES	0.0440769 in	n/in	εcu*((d-c)/c)
Ductility Check		PASS		
DCR				
Reduced Nominal Moment	φMn	5910.6748 k-	-ft	φAsfy(d-a/2)
DCR	DCR	0.7174088		Mu/φMn
DCR Check		PASS		DCR<1 (typ values between 0.9 and 1

Table C: ACI Compliance Checks for Box Girder experiencing positive bending.

Top Base Base Web Height Effective depth Length General Strength Yield Strength Type of Concrete  Load Combination 1 Wu1 Load Combination 2 Governing Load Combination Moment Mu	Technology	Normal Weight or Light Weight  1.4WD 1.2WD+1.6WL  Mu/\$\phi\$ =0.9*d Mn/Z
Base Web Height Effective depth Length Length Dead Load Live Load WD Live Load WL  Compressive Strength Yield Strength Type of Concrete  Load Combination 1 Load Combination 2 Governing Load Combination Moment Mu Φ Nominal Moment Mn Z T Area of Steel Required As,req  Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel Minimum width Section Width Check Minimum Section Width Check Minimum Section Height Check Tebar diameter strirup diameter strirup diameter ds minimum Clear cover Minimum Height Check Minimum Reinforcement Check Minimum Reinforcement Check Percent of Steel Percent of Steel Minimum Reinforcement Check Vield Strain of Steel Fecultity Check Fecultity C	72 in 75 in 75 in 72 in 80 ft 32.05 k/ft 13.84 k/ft  Materials 5 ksi 60 ksi  Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2  Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Height h Effective depth d Length L Dead Load WD Live Load WL  Compressive Strength Yield Strength Type of Concrete  Load Combination 1 Wu1 Load Combination 2 Wu2 Governing Load Combination Moment Mu ф Nominal Moment Mn Z T Area of Steel Required As, req  Select Steel Reinforcement Check Number of rebars n Rebar cross-sectional area Area of Steel As Reinforcement Check Minimum width bmin Section Width Check Minimum Section Height Check rebar diameter ds strirup diameter ds strirup diameter ds minimum Lear cover Minimum Height Check Percent of Steel pmin Reinforcement Check Minimum Reinforcement Check Minimum Reinforcement Check Percent of Steel pmin Reinforcement Check Vield Strain of Steel ECU a	75 in 72 in 80 ft 32.05 k/ft 13.84 k/ft  Materials 5 ksi 60 ksi  Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2  Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Effective depth Length Vield Strength Yield Strength Type of Concrete  Load Combination 1 Wu1 Load Combination Moment Mu Φ Nominal Moment Mn Z T Area of Steel Required As, req  Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Section Width Check Minimum Section Width Check rebar diameter strirup diameter db strirup diameter dc dc Minimum Height Check Percent of Steel Pomin Reinforcement Check Vield Strain of Steel ECU a	72 in 80 ft 32.05 k/ft 13.84 k/ft  Materials 5 ksi 60 ksi  Structural Analysis 44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Length Dead Load WD Live Load WL  Compressive Strength Yield Strength Type of Concrete  Load Combination 1 Wu1 Load Combination 2 Wu2 Governing Load Combination Moment Mu Pominal Moment Mn Z T T Area of Steel Required As, req  Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel As Reinforcement Check Minimum Width Check Minimum Width Check Minimum Height Check Tebar diameter Strirup diameter Strirup diameter Minimum Height Check Minimum Height Check Minimum Height Check Minimum Reinforcement Check Dercent of Steel Pomin Reinforcement Check Outtility Check Yield Strain of Steel ECU a	80 ft 32.05 k/ft 13.84 k/ft  Materials 5 ksi 60 ksi  Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2  Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Dead Load Live Load  Compressive Strength Yield Strength Type of Concrete  Load Combination 1 Load Combination 2  Governing Load Combination Moment Mu	32.05 k/ft 13.84 k/ft  Materials 5 ksi 60 ksi  Structural Analysis 44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Live Load WL  Compressive Strength Yield Strength Type of Concrete  Load Combination 1 Wu1 Load Combination 2 Wu2 Governing Load Combination Moment Mu  Physical Programmer Compressive Steel Required As, req  Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Section Width Check Minimum Width Section Width Check Minimum Section Height Check Tebar diameter distribution of Steel Strirup diameter minimum clear cover Minimum Height Check Minimum Reinforcement Check Minimum Percent of Steel pmin Reinforcement Check Minimum Percent of Steel pmin Reinforcement Check Minimum Percent of Steel pmin Reinforcement Check Minimum Percent of Steel ECU  A SECULATION OF STEEL ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM Reinforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM REINforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM REINforcement Check  Minimum Percent of Steel ECU  B COMPRESSIVE STEEL PM PMINIMUM REINforcement Check  Minimum	13.84 k/ft  Materials  5 ksi 60 ksi  Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2  Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Compressive Strength Yield Strength Type of Concrete  Load Combination 1 Load Combination 2 Governing Load Combination Moment Mu	Materials 5 ksi 60 ksi  Structural Analysis 44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Yield Strength Type of Concrete  Load Combination 1 Load Combination 2 Wu2 Governing Load Combination  Moment Mu	5 ksi 60 ksi  Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2  Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Yield Strength Type of Concrete  Load Combination 1 Load Combination 2 Wu2 Governing Load Combination  Moment Mu	60 ksi  Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2  Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi =0.9*d
Type of Concrete  Load Combination 1 Load Combination 2 Wu2 Governing Load Combination  Moment Mu	Structural Analysis  44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9  4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi = 0.9*d
Load Combination 1 Load Combination 2 Wu2 Governing Load Combination  Moment Mu	44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	1.4WD 1.2WD+1.6WL Mu/\phi = 0.9*d
Load Combination 2  Governing Load Combination  Moment  Mu	44.87 k/ft 60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	1.2WD+1.6WL $Mu/\phi = 0.9*d$
Load Combination 2  Governing Load Combination  Moment  Mu	60.604 k/ft 60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	$1.2WD+1.6WL$ $Mu/\varphi$ =0.9*d
Governing Load Combination  Moment  Mu	60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	Mu/φ =0.9*d
Moment Mu φ Nominal Moment Mn Z T Area of Steel Required As,req  Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Width Check Minimum Width Check Minimum Section Width Check rebar diameter strirup diameter strirup diameter minimum clear cover Minimum Height Check Minimum Height Check Minimum Reinforcement Check Percent of Steel Percent of Steel Minimum Reinforcement Check Vield Strain of Steel ECU a	60.604 k/ft 4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	Mu/φ =0.9*d
Moment Mu  \$\phi\$ Nominal Moment Mn  \$Z\$  \$T\$  Area of Steel Required As,req  Select Steel Reinforcement Check  Number of rebars  Rebar cross-sectional area  Area of Steel  Reinforcement Check  Minimum Section Width Check  Minimum Section Width Check  Minimum Section Height Check  rebar diameter  strirup diameter  minimum clear cover  Minimum Height Check  Minimum Height Check  Percent of Steel  Percent of Steel  Reinforcement Check  Vield Strain of Steel  ECU  a	4240.37 k-ft 0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	=0.9*d
Nominal Moment  Area of Steel Required  Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Section Width Check Minimum Width Section Width Check Minimum Section Height Check rebar diameter strirup diameter minimum clear cover Minimum Height Check Percent of Steel Minimum Reinforcement Check Percent of Steel Minimum Reinforcement Check Vield Strain of Steel ECU a	0.9 4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	=0.9*d
Nominal Moment  Z T  Area of Steel Required  Select Steel Reinforcement Check  Number of rebars Rebar cross-sectional area Area of Steel Reinforcement Check  Minimum Section Width Check  Minimum Section Width Check  Minimum Section Height Check  rebar diameter strirup diameter strirup diameter minimum clear cover Minimum Height  Minimum Height Check  Percent of Steel Percent of Steel Minimum Percent of Steel Minimum Percent of Steel Suctility Check  Yield Strain of Steel ECU a	4711.5222 k-ft 64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	=0.9*d
Area of Steel Required As, req  Select Steel Reinforcement Check Number of rebars n Rebar cross-sectional area Area of Steel As Reinforcement Check Minimum Section Width Check Minimum Width Section Width Check Minimum Section Height Check rebar diameter strirup diameter ds strirup diameter ds minimum clear cover Minimum Height Check Minimum Height Check Minimum Height Check Minimum Reinforcement Check Percent of Steel pmin Reinforcement Check Vield Strain of Steel ECU a	64.8 in 872.50412 kip 14.5 in^2 Checks and Drawings	=0.9*d
Area of Steel Required As,req  Select Steel Reinforcement Check Number of rebars n Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Section Width Check Minimum width Section Width Check Minimum Section Height Check rebar diameter strirup diameter ds strirup diameter minimum clear cover Minimum Height Check Minimum Height Check Minimum Height Check Minimum Reinforcement Check Percent of Steel pmin Reinforcement Check Ouctility Check Yield Strain of Steel ECU a	872.50412 kip 14.5 in^2 Checks and Drawings	
Area of Steel Required As,req  Select Steel Reinforcement Check Number of rebars n Rebar cross-sectional area Area of Steel As  Reinforcement Check Minimum Section Width Check Minimum Section Width Check Minimum Section Height Check rebar diameter strirup diameter ds strirup diameter minimum clear cover Minimum Height Check Minimum Height Check Minimum Reinforcement Check Percent of Steel pmin Reinforcement Check Ouctility Check Yield Strain of Steel ECU a	14.5 in^2 Checks and Drawings  12 1.56 in^2	MIZ
Select Steel Reinforcement Check Number of rebars Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Section Width Check Minimum Width Section Width Check Minimum Section Height Check rebar diameter strirup diameter minimum clear cover Minimum Height Minimum Height Check Percent of Steel Minimum Percen	Checks and Drawings  12 1.56 in^2	
Number of rebars n Rebar cross-sectional area Area of Steel As Reinforcement Check Minimum Section Width Check Minimum Section Width Check Minimum Section Height Check rebar diameter strirup diameter ds strirup diameter minimum clear cover Minimum Height Check Minimum Height Check Minimum Height Check Minimum Reinforcement Check Percent of Steel ρ pmin Reinforcement Check Ouctility Check Yield Strain of Steel ECU a	12 1.56 in^2	
Number of rebars n Rebar cross-sectional area Area of Steel As Reinforcement Check Minimum Section Width Check Minimum Section Width Check Minimum Section Height Check rebar diameter strirup diameter ds strirup diameter minimum clear cover Minimum Height Check Minimum Height Check Minimum Height Check Minimum Height Check Minimum Reinforcement Check Percent of Steel ρmin Reinforcement Check Ouctility Check Yield Strain of Steel ECU a	1.56 in^2	
Rebar cross-sectional area Area of Steel Reinforcement Check Minimum Section Width Check Minimum Width Section Width Check Minimum Section Height Check rebar diameter strirup diameter strirup diameter minimum clear cover Minimum Height Minimum Height Check Minimum Reinforcement Check Percent of Steel Minimum Percent of Steel Reinforcement Check Vield Strain of Steel ECU a	1.56 in^2	
Area of Steel As  Reinforcement Check  Minimum Section Width Check  Minimum width Section Width Check  Minimum Section Height Check  rebar diameter ds strirup diameter aminimum clear cover Minimum Height Check  Minimum Height Check  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel pmin Reinforcement Check  Ductility Check  Yield Strain of Steel ECU a		
Reinforcement Check  Minimum Section Width Check  Minimum width Section Width Check  Minimum Section Height Check  rebar diameter strirup diameter ds strirup diameter minimum clear cover Minimum Height Minimum Height Check  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel Percent of Steel Reinforcement Check  Vield Strain of Steel CC pmin Reinforcement Check  ECU a		
Minimum Section Width Check  Minimum width Section Width Check  Minimum Section Height Check  rebar diameter strirup diameter ds Strirup diameter minimum clear cover Minimum Height Minimum Height Check  Minimum Reinforcement Check  Percent of Steel Percent of Steel Reinforcement Check  Vield Strain of Steel CCC pmin Reinforcement Check  ECU a	18.72 in^2 PASS	
Minimum width Section Width Check  Minimum Section Height Check rebar diameter strirup diameter ds strirup diameter minimum clear cover Minimum Height Minimum Height Check  Minimum Reinforcement Check Percent of Steel Percent of Steel Reinforcement Check  Ductility Check Yield Strain of Steel ECU a	PA33	
Section Width Check  Minimum Section Height Check  rebar diameter db strirup diameter ds minimum clear cover Minimum Height Check  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel pmin Reinforcement Check  Ductility Check  Yield Strain of Steel ECU a	36.62 in	From Table 5
Minimum Section Height Check  rebar diameter db strirup diameter ds minimum clear cover Cc Minimum Height Check  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel pmin Reinforcement Check  Ductility Check  Yield Strain of Steel & ECU a		From Table 5
rebar diameter db strirup diameter ds ds minimum clear cover Cc Minimum Height Check  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel pmin Reinforcement Check  Ductility Check  Yield Strain of Steel & ECU a	PASS	
strirup diameter ds minimum clear cover Cc Minimum Height hmin Minimum Height Check  Minimum Reinforcement Check Percent of Steel pmin Reinforcement Check  Ouctility Check  Yield Strain of Steel ECU a		·
minimum clear cover Minimum Height hmin  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel Minimum Percent of Steel Reinforcement Check  Ductility Check  Yield Strain of Steel ECU a	1.128 in	from Table 5
Minimum Height hmin  Minimum Height Check  Minimum Reinforcement Check  Percent of Steel pmin  Reinforcement Check  Ductility Check  Yield Strain of Steel & ECU  a	0.375 in	
Minimum Height Check  Minimum Reinforcement Check  Percent of Steel ρ  Minimum Percent of Steel ρmin  Reinforcement Check  Ductility Check  Yield Strain of Steel εCU  a	1.5 in	
Minimum Reinforcement Check Percent of Steel ρ Minimum Percent of Steel ρmin Reinforcement Check  Ductility Check  Yield Strain of Steel εCU a	74.439 in	hmin=d+db/2+ds+Cc
Percent of Steel p Minimum Percent of Steel pmin Reinforcement Check  Ductility Check  Yield Strain of Steel ECU a	PASS	
Minimum Percent of Steel pmin Reinforcement Check  Ductility Check  Yield Strain of Steel & ECU a		Briff van 10
Reinforcement Check  Ductility Check  Yield Strain of Steel ECU a	0.0036111	As/bd
Ductility Check  Yield Strain of Steel ECU a	0.0035	from Table A.7
Yield Strain of Steel ECU	PASS	
а	and the state of t	
	0.003 in/in	
	3.6705882 in	As*fy/0.85*f'c*b
β	0.0	From Table A.7
c	0.8	а/β
ES	4.5882353 in	εcu*((d-c)/c)
Ductility Check	4.5882353 in	((4))
DCR	4.5882353 in 0.0440769 in/in	
	4.5882353 in	
	4.5882353 in 0.0440769 in/in PASS	machild al2)
DCR DCR DCR Check	4.5882353 in 0.0440769 in/in	φAsfy(d-a/2) Mu/φMn

Table D: ACI Compliance Checks for Box Girder experiencing negative bending.

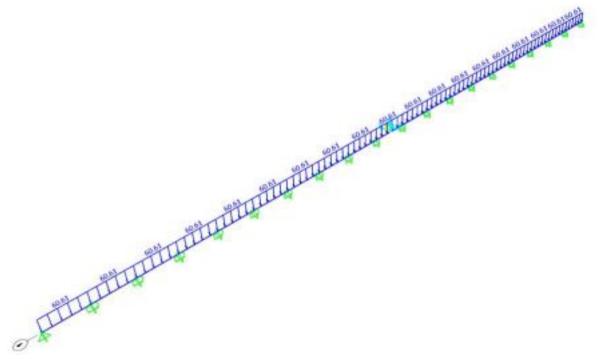


Figure 1 - SAP Model with Loading in k/feet

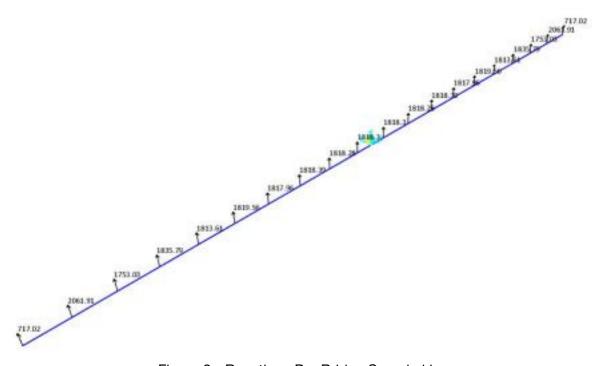


Figure 2 - Reactions Per Bridge Span in kips

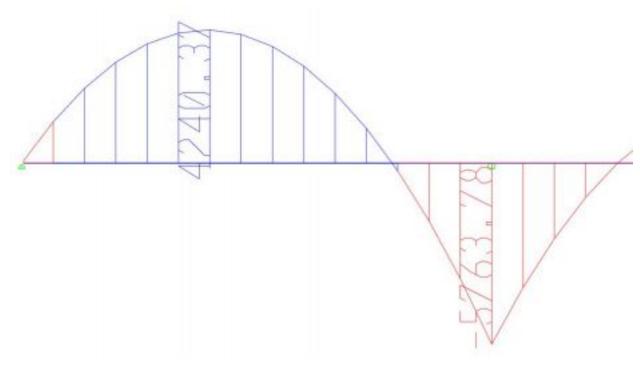
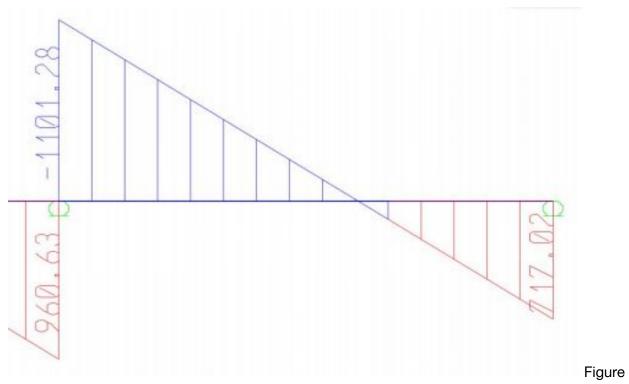


Figure 3 - Maximum Positive and Negative Bending Moments in kip-feet



4 - Maximum Shear in kips

## **Design Report - Overpass**

All design and analysis will be based on AASHTO LRFD highway specifications and the ACI code to have the best strength design limits for this structure. LRFD load combinations will be used to help determine loads of worst-case scenarios so that the design can avoid and mitigate around those extreme events. Below in table (1-10) and figures 1-8, illustrates and calculates the different scenarios for dead, live and ACI calculations for rebar. In another document, I will discuss the other loading that is needed to take notice as well for constructing the abutments and needed number of piles.

The Overpass structure is built to reduce the traffic flow and allow vehicles to have a smooth transition on and off the highway in San Elijo. Based on the preliminary design, it was decided to use a box girder structure for the two different overpass bridges based on the length, curvature and time efficiency. One bridge is estimated to have 120 feet long with 93 feet span width. This overpass bridge consists of two different box bridges connected with a closure pour. The curved bridge has a superelevation of 2 degrees due to a wide curve of 132 feet long and 33 and a half feet for the span width. The Box girder design allows the two bridges to hold their own dead and live load when traffic presumes. This method of construction for the box girder will be prefabricated by another contractor to create a more time efficiency in focusing on the platforms and abutments that will be the foundation for the overpass bridges.

#### Construction

To start the project as a whole, there will be an excavation process to allow a road to follow underneath both overpasses. This road will allow drivers to have direct access to the two HOV lanes. The soil that will be lifted up to have a gradual decline underneath the overpass will be used as the elevation foundation for both bridges to execute a higher elevation. After the settlement of the soil, the piles will be drilled into the ground, once it hits the bedrock. There will be 12 of piles at each abutment at a length of 150 feet for the curved abutment. For the Straight abutment, there will be 14 piles at a length of 150 feet. There will be two layers of piling to help with overcoming the torsion and moment that the bridges will have due to the dead and live loading.

The abutments will be constructed before the box girders arrive to ensure a solid foundation that will carry the vertical and horizontal loads. During the first construction phase of the straight overpass bridge there will not be a lane closure, due to this box girder will only be expanding the highway and not have any interference with the traffic. As soon as the right box girder has been placed, the traffic will be moved to the right side to allow the construction of the HOV lanes (left side) to begin. There will be a lane closure pour to connect both box girders together along with a barrier to separate the

two sides. As for the curve overpass bridge, the box girder will be in place in one piece as soon as the abutments have been constructed. The wingwalls will be also worked on as a finished phase of the two bridges. This will allow a smooth transition from road to bridge.

#### Load Analysis

Dead load is the constant loading caused by the weight and density of the materials used, such as concrete, and steel. To find the dead load of a structure, or slab, the density of the material needs to be multiplied by the depth of the slab. Now to find the UDL (uniformly distributed load) of the slab, it's taking the dead load and multiplying by the area of the slab. Both overpass bridges have a concrete slab of 8 inches. This will allow the bridges to have greater durability and longer life cycle since it is near the ocean. The calculations are shown in Tables 1, 2, 4 and 5.

Just like the dead load, the live load takes the loading caused by the weight and area, but this time in motion. For bridges, and roadways, the moving object would be the vehicles and the amount of traffic expected to travel. Now, pedestrian bridges take into account vehicles, but as well as people. In the overpass structure, there are no sidewalks, therefore no people. When calculating the live load, it is always best to consider the worst scenario for a bridge. This would be to measure the weight of a Semi-truck of 20,000 pound or even more for each lane to see if it still passes. The calculations for these two overpasses are in Table 3 and Table 6.

Curve Overpass						
Dead Load	97.53 lb/ft					
Live Load	1280 lb/ft					
Straight O	verpass					
Dead Load	25592 lb/ft					
Live Load	3840 lb/ft					

Table 1: Dead load and Live Loading for Overpass

#### **ACI Compliance Checks**

In the ACI Compliance Check, it calculates everything that the design is shown in the AutoCAD to ensure that it works for the dead loads. This also allows to figure out the required amount of rebar for the additional reinforcements. All calculations for the two overpass bridges

are in Table 7 and Table 8. For the Curve and Straight Overpass, there is accountability for the length, base of the flanges and walls, dead load and live load. With the chosen load combination, W(u), the moment can be found for the walls and slabs. Area of steel required will be based on the moment found. The area of steel required is only the minimum requirement for the reinforcement needed. When calculating the number of rebar needed to reach the required area of steel, it's recommended to get as close to the value of the required. Therefore, there will not be an excess of rebar, that could potentially harm the structure. For the Clear cover of the bottom flange of both overpass bridges, it will have a minimum of 4 inches, due to the environment the cement is constantly exposed to.

#### Wind Loads

When calculating wind loads, it's important to know the locations of the project. The two overpasses are in San Elijo, close to the coastlines. With that notice, it puts it in a higher risk category due to possible earthquakes, hurricanes and coastal storms that could potentially damage the bridges and put citizens at risk. This put the overpass bridges at a Risk Category 3 due to the potential lives that could be effected. The Basic wind speed for this location is 100 mph, given by the ASCE Standard in Figure 26.5-1C.

#### SAP Modeling

The Sap modeling provided in this design report, represents the dead and live loading on the bridge. It allows us to visualize how the bridge will be affected with the loading applied to it and calculate the moment and shear of the bridges. as well as, combine and ensure that the moment of the bridges are the same or similar to the ACI Compliance Checks. The illustration is shown in Figure 1 through 8.

	Job No.		Sheet:		Rev:	
		San Elijo		placement Brid		
	Subject	Curve Ov				
	OII .		Made	Nadia Mana	Date	10/18/2020
	Client		By: Check	Nadia Manzur	Date:	10/18/2020
			By:		Date:	
	Calc	ulation She	et for Dead	Loads		
Dead load = Density *		alacion Sile	et for beda	20443		
Top Slab						
Desity of Concrete		150 pcf				
Slab Thickness		8 inch	es			
Width		33.5 feet				
Length		132 Feet				
Area		4422 f^2				
Volume		2948 f^3				
Dead Load	44	12200 lb				
Datta Clab						
Bottom Slab		150 n of				
Desity of Concrete Slab Thickness		150 pcf 8 inch	0.5			
Width		20.7 feet				
Length		132 Feet				
Length		132 1 660				
Area		732.4 f^2				
Volume		821.6 f^3				
Dead Load	27	73240 lb				
Walls (3)						
Desity of Concrete		150 pcf				
Slab Thickness		54 inch	es			
Width		0.67 feet				
Length		132 Feet				
Area		88 f^2				
Volume		396 f^3				
Dead Load	5	59400 lb				
#of Coumns		3				
Column Dead Load	17	78200 lb				
		CONTRACTOR OF THE STATE OF				
Ī						

Table 1: Dead Load of Curve Over Pass

	Job No.		Sheet:		Rev:	
	Job Title	San Elijo	Lagoon Re	placement Brid	ge	
	Subject	Curve Ov				
			Mada		· ·	
	Client		Made By:	Nadia Manzur	Date:	10/18/2020
	Client		Check	radia manzar	Date.	10/10/2020
			By:		Date:	
		Calcula		for Dead Loads		
Dead load = Density *	Volume	Calcula	ition sheet	ioi Dead Loads		
Ashphalt	Volume					
Depth	3	in				
length	132					
width	29.5					
weight of asphaly	130					
Volume	973.5					
Dead load of Aspalt	126555	lb				
Parapets						
Depth	4.5	ft				
Length	132	ft				
Width	1.5					
Density	150	pcf				
quanity	2					
Malaura a	001					
Volume	891	11_				
Total Dead load for Pa						
	267.3	KIPS				
Total Dead Load						
	1287495	lb				
	1287.50					
	0.291					
w (d)	9753.75	lb/ft				

Table 2: Continue of Dead Load Calculations of Curve Overpass

	Job No.		Sheet:		Rev:		
	Job Title	San Elijo	Lagoon Re	placement Brid			
	Subject Curve Ove		erpass				
			Made				
	Client		By:	Nadia Manzur	Date:	10/18/2020	
			Check		Datas		
			By:		Date:		
Live Load = Area * Liv	e loa <mark>d</mark> ing tr		ulation Shee	et for Live Loads			
AA	H-20						
Number of Lanes		2					
Mutiple Presense Fac		L					
Total Vehicle weight		2 kips					
Design lane Load		1 Klf					
Loading Cases							
Loaded Length	132	2 ft					
Loaded Width	24	1 ft					
Live Load Factor	640	plf					
Live Load of Slab	168960	) Lb					
1/2000	168.96						
w(I)	1280	) lb/ft					

Table 3: Live Load Calculations for Curve Overpass

	Job No.		Sheet:		Rev:	
	Job Title	San Elijo	Lagoon Re	eplacement Brid	ge	
	Subject	Straight C				
	5-700-800		Made			
	Client		By:	Nadia Manzur	Date:	10/26/2020
			Check		Data	
			By:		Date:	
		Calcu	ılation Sheet	for Dead Loads		
Dead load = Densit	ty * Volume			THE WAY TO SEE THE PARTY OF		
Top Slab #1			Top Sla	10 ( C C C C C C C C C C C C C C C C C C		
Desity of Concrete	150 pc	:f	Desity	of Concrete	150 pcf	
Slab Thickness	8 in		Slab Th	ickness	8 in	
Width	62 ft		Width		30 ft	
Length	120 ft		Length		120 ft	
union in the second sec						
Area	7440 ft/	^2	Area		3600 ft^2	
Volume	4960 ft/	/3	Volume	è	2400 ft^3	
Dead Load	744000 lb		Dead Lo	oad 3	860000 lb	
Bottom Slab#1			Bottom	n Slab#2		
Desity of Concrete	150 pc	:f	Desity	of Concrete	150 pcf	
Slab Thickness	8 in		Slab Th	ickness	8 in	
Width	51 ft		Width		18 ft	
Length	120 ft		Length		120 ft	
Area	6120 ft/	<b>^2</b>	Area		2160 ft^2	
Volume	4080 ft/	<b>\</b> 3	Volume	9	1440 ft^3	
Dead Load	612000 lb		Dead Lo		216000 lb	
Walls Abutment #1			Walls /	Abutment #2		
Desity of Concrete	150 pc	:f	Desity	of Concrete	150 pcf	
Slab Thickness	54 in		eranger carrier (Fig.	ickness	54 in	
Width	0.67 ft		Width		0.67 ft	
Length	120 ft		Length		120 ft	
Area	80 f^2	2	Area		80 f^2	
Volume	360 f^3		Volume	3	360 f^3	
Dead Load	54000 lb		Dead Lo		54000 lb	
# of Walls			# of Wa		3	
the animal contraction of the second	6 224000 lb					
Column Dead Load	324000 lb		Column	n Dead Load 1	.62000 lb	

Table 4: Dead Load Calculations for Straight Overpass

	Job No.		Sheet:		Rev:	
	Job Title	San Elijo	Lagoon Re	placement Brid	ge	,
	Subject	Straight C	Overpass			
	1000					
	Client		Made By:	Nadia Manzur	Date:	10/26/2020
	023 - 345		Check By:		Date:	
		C	Calculation Sh	neet for Dead Loads	5	
Ashphalt #1			Ashpha	lt #2		
Depth	3 i	n	Depth		3	in
length	120 f	t	length		120	ft
width	48 1	t	width		26	ft
weight of asphaly	130	ocf	weight	of asphaly	130	pcf
Volume	1440		Volume		780	
Dead load of Aspalt	187200 l	b	Dead lo	ad of Aspalt	101400	lb
Parapets			Parape	ts		
Depth	4.5 1	t	Depth		4.5	ft
Length	120 f	t	Length		120	ft
Width	1.5 1	t	Width		1.5	ft
Density	150	ocf	Density		150	pcf
quanity	1.5		quanity		1.5	
Volume	810		Volume	2	810	
Total Dead load for Para	182250 l	b	Total D	ead load for Parap	182250	lb
	182.25 l	kips			182.25	kips
Total Dead Load	2049450 I	b	Total D	ead Load	1021650	lb
	2049.45				2049.45	kips
	0.275464				0.559959	
w (d)	17078.75 l	b/ft	w (d)		8513.75	lb/ft
Total Dead Load	3071100 l	b				
	3071.1					
	0.276676	1 -				
w (d)	25592.5	b/ft				

Table 5: Continue of Dead Load Calculations for Straight Overpass

	Job No.		Sheet:		Rev:	
	Job Title	San Elijo	Lagoon Re	placement Brid	ge	
	Subject	Straight (	Overpass			
	Client		Made By:	Nadia Manzur	Date:	10/26/2020
			Check By:		Date:	
		Calcul	ation Sheet fo	or Live Loads		
Live Load = Area * Live	e loading traffic					
Live Load for Abutme	The state of the s					
AA	H-20					
Number of Lanes	4					
Loaded Length	120					
Loaded Width	48	ft				
Live Load Factor	640	plf				
Live Load of Slab	307200					
12.00	307.2					
w(I)	2560	lb/ft				
Live Load for Abutme	nt #2					
Live Load for Abutme	nt #2 H-20					
AA	H-20					
AA Number of Lanes	H-20 2 120					
AA Number of Lanes Loaded Length	H-20 2 120 24	ft				
AA Number of Lanes Loaded Length Loaded Width	H-20 2 120 24 640 153600	ft ft plf				
AA Number of Lanes Loaded Length Loaded Width Live Load Factor  Live Load of Slab	H-20 2 120 24 640 153600	ft ft plf Lb Kf				
AA Number of Lanes Loaded Length Loaded Width Live Load Factor	H-20 2 120 24 640 153600	ft ft plf				
AA Number of Lanes Loaded Length Loaded Width Live Load Factor  Live Load of Slab	H-20 2 120 24 640 153600	oft ft oplf Lb Kf olb/ft				
AA Number of Lanes Loaded Length Loaded Width Live Load Factor  Live Load of Slab  w(I)	H-20 24 640 153600 153.6	oft ft plf Lb Kf lb/ft				

Table 6: Live Load Calculations for Straight Overpass

ACI	Compli	ance Checks For Curv	ve Overpass
30 Ga S 10 5 Ga		Geometry	1
Base of walls	b(w)	24 in	
base of Bottom Flange	b	248.4 in	
Base of Flange	b(e)	402 in	
Height	h	70 in	
Effective depth	d	66 in	
Length	L	132 ft	
Self Dead Load	WSDL	9.75 K/ft	
Live Load	WL	1.28 K/ft	
		Materials	
Compressive Strength	fc	6 psi	
Yield Strength	fy	60 ksi	
Type of Concrete		150 pcf	Normal Weight or Light Weight
		Structural Analysis	
Dead Load due to Self Weight	WSW	1.75 k/ft	((b * h)/144) *150pcf
	WD	11.50 k/ft	wsw + wsdl
Load Combination 1	Wu1	16.10 k/ft	1.4WD
Load Combination 2	Wu2	15.85 k/ft	1.2WD+1.6WL
Governing Load Combination		16.10 k/ft	
Moment	Mu	35065.80 k-ft	((Wu)(L^2))/8 (simply supported)
	ф	0.90	
Nominal Moment	Mn	38962.00 k-ft	Mu/ф
	Z	4.95 ft	=0.9*d
	T	7871.11 kip	Mn/Z
Area of Steel Required	As,req	131.19 in^2	
1		Checks and Drawings	
Select Steel Reinforcement Check			
Number of rebars	n	105	
Rebar cross-sectional area	A	1.27 in^2	
Area of Steel	As	133.35 in^2	
Reinforcement Check		PASS	
Minimum Section Width Check		V MC WYMAN .	
Minimum width	bmin	213.25 in	From Table 5
Section Width Check		PASS	
Minimum Section Height Check		1017.7	
rebar diameter	db	1.27 in	from Table 5
strirrup diameter	ds	0.375 in	
minimum clear cover	Cc	2.5 in	
Minimum Height	hmin	69.51 in	hmin=d+db/2+ds+Cc
Minimum Height Check		PASS	
Minimum Reinforcement Check			
The state of the s	ρ	0.0842	ρ/bd
	ρmin	0.0285	from Table A.7
Reinforcement Check	p	PASS	
Ductility Check		12.00	
Yield Strain of Steel	εcu	0.003 in/in	
nois of all of otech	a	6.32 in	As*fy/0.85*f'c*b
	β	0.75	Fromt Table A.7
	C	8.42 in	a/β
		0.02 in/in	ecu*((d-c)/c)
Ductility Check	ES	PASS PASS	ccu ((u-c//c/
OCR Ductility Check		1 A33	
/CII	V-V/=1-000		
Peduced Naminal Managet		27710 0	
Reduced Nominal Moment DCR	φMn DCR	37710.0 0.930	φAsfy(d-a/2) Mu/φMn

Table 7: ACI Compliance Calculation Check for Curve Overpass

	ACI	Compliance	Checks	
		Geometry		
Base of walls	b(w)	72	in	
base of Bottom Flange	b	828	in	
Base of Top Flange	b(e)	1116	in	
Height	h	70	in	
Effective depth	d	66	in	
Length	L	120	ft	
Self Dead Load	WSDL	25.6	K/ft	
Live Load	WL	3.84	K/ft	
		Materials		
Compressive Strength	fc	6	psi	
Yield Strength	fy	60	ksi	
Type of Concrete	1 P2	150	pcf	Normal Weight or Light Weight
		Structural Analy		
Dead Load due to Self Weight	WSW	5.25	k/ft	((b * h)/144) *150pcf
	WD	30.84	k/ft	wsw + wsdl
Load Combination 1	Wu1	43.18	k/ft	1.4WD
Load Combination 2	Wu2	43.16	k/ft	1.2WD+1.6WL
Governing Load Combination		43.18	k/ft	
Moment	Mu	77723.10	k-ft	((Wu)(L^2))/8 (simply supported)
	Ó	0.9		
Nominal Moment	Mn	86359.00	k-ft	Mu/ф
	Z	4.95	ft	=0.9*d
	T	17446.26	kip	Mn/Z
Area of Steel Required	As,req	290.77	-	
		hecks and Draw		
Select Steel Reinforcement Check				
Number of rebars	n	235		
Rebar cross-sectional area	A	1.27	in^2	
Area of Steel	As	298.45		
Reinforcement Check		PASS		
Minimum Section Width Check		.,,,,,		
Minimum width	bmin	599.61	in	From Table 5
Section Width Check	Dillill	PASS	***	Trotti lable 3
Minimum Section Height Check				
rebar diameter	db	1.27	in	from Table 5
strirrup diameter	ds	0.375		Total lubic 5
minimum clear cover	Cc	2.5		
Minimum Height	hmin	69.51		hmin=d+db/2+ds+Cc
Minimum Height Check	11111111	PASS PASS	311	TITIIIII—UTUD/ ZTUSTCC
Minimum Reinforcement Check		rMJJ		
viiiiiiiidiii keiiiloi cement Check	-	0.052		ρ/bd
	ρ omin	0.063 0.0285		from Table A.7
Reinforcement Check	ρmin	PASS		HOIII lable A./
		PASS		
Ouctility Check				
Yield Strain of Steel	ECU	0.003		* *f /o or*f *!
	а	4.24	in	As*fy/0.85*f'c*b
	β	0.75		Fromt Table A.7
	С	5.65	in	a/β
	es	0.03	in/in	εcu*((d-c)/c)
Ductility Check	G8/64	PASS		0.00 to 0.000 to 0.00
OCR				
Reduced Nominal Moment	φMn	85792.1		φAsfy(d-a/2)
DCR	DCR	0.906		Mu/φMn
OCR Check		PASS		DCR<1 (typ values between 0.9 an

Table 8: ACI Compliance Calculation Check for Straight Overpass

	CI Compili	ance Checks For		TOP State
T 0.0		Material and Loadin		
Type of Concrete			pef	
	L		ft	
	fc		ksi	
	fy		ksi	
	h(min)		in	one way- simply supported slab
assumed	b		in	
Actual	ь	402	in	
Primiary reinforcement		#6		
	db	0.75	in	
	Ab	0.44	in^2	
	d	6.875		h- (3/4) - (db/2)
Self-weight dead load	Swslab	100	lb/ft	1-7 7 1-7-7
	WSD	189		
	WD		lb/ft	
	WL	38.21		
	VIL	Structural Analysis		
Landing Combination	1474			1.4 *WD
Loading Combination	Wu1	264.6		
6	Wu2	325.736	The state of the s	1.2*WD + 1.6WL
Governing Moment	Wu	325.736	the latest the second	
Moment	Mu		k-ft/ft	Wu * L^2 / 8
	ф	0.90		
	R	0.008		Mu/phi *b* d^2
required *	As >=	0.011		(.85*f'c*b*d)/fy)*(1-sqrt(1- (2*R/.85f'c)
#6 spacing	Ab	0.44	in^2	
	S	8.00	in	(Ab+12)/ A(required)
	As(min)	0.1728		.0018 (b*h)
	As/ft	0.660		Ab*12/ spacing
Reinfo	cement Check	Pass		The same of the sa
incili o		ect Steel Reinforcemer	t Check	
	Rebar	#6	IL CHECK	
		100.00	:- 40	
	As	0.660		
	Diameter	0.75		
Spacing	S	8.00		
	n	46		
	Total length	395	in	
Total Width of Span	В	402	in	
Percent Error		1.9	%	
Max Steel R	einforcement	Pass		0%-5% error
		Check and Drawing	s	
Moment	Mu	4.07		((Wu)(L^2))/8 (simply supported)
	ó	0.90		
Nominal Moment	Mn	4.52	k-ft	Mu/o
Ivoliniai women	Z	0.52		=0.9*d
	T	8.77		Mn/Z
Annual Secol Provinced			in^2	NIII/Z
Area of Steel Required	As,req			
Number of rebars	n	46		
Rebar cross-sectional area	A		in^2	
Area of Steel	As	20.24	ın^2	
Reinforcement Check		PASS		
Minimum Section Width Check				
Minimum width	bmin	84	in	From Table 5
Section Width Check		PASS		
Minimum Section Height Check				11.0
rebar diameter	db	0.75	in	from Table 5
strirrup diameter	ds	0.375		
minimum clear cover	Cc	2.5		
				hmin-dudh/2udayC-
Minimum Height	hmin	3.69	111	hmin=d+db/2+ds+Cc
Minimum Height Check		PASS		
Minimum Reinforcement Check		- Office and the second		
	ρ	11.2975		ρ/bd
	pmin	0.0285		from Table A.7
Reinforcement Check	20	PASS		
Ductility Check				
Yield Strain of Steel	ECU	0.003	in/in	
new strain or steel	3	0.59		As*fy/0.85*f'c*b
		0.39		
	β			Fromt Table A.7
	С	0.79		a/β
	٤٤	0.02	in/in	εcu*((d-c)/c)
Ductility Check		PASS		
Ductility check				
pMn> Mu				
AND	Mu	4.07	k-ft	
pMn> Mu	Mu øMn	4.07 599.2		φAsfy(d-a/2)

Table 9: ACI Requirement for Curve Top Slab

	ACI Comp			irve Overpass
		Material and		
Type of Concrete		150		
Estimate	L	10		
	fc		ksi	
	fy		ksi	
7.00 1 to 10.00 1 (1.00 to 10.00 to 10.	h(min)		in	one way- simply supported slab
assumed	b	12		
Actual	b	1116	in	
Primiary reinforcement		#6		
	db	0.75		
- · ·	Ab		in^2	
Estimate	d	4.9	11 (6)	h-1.1
Self-weight dead load	Swslab		lb/ft	
	WSD	159.15	11 (6)	
	WD	159.15		
	WL	41.73		
1 0 1:	147.4	Structural A		4 4 114/15
Loading Combination	Wu1	222.81		1.4 *WD
	Wu2	289.578		1.2*WD + 1.6WL
Governing Moment		289.578		M( * 1 A2 / 2
Moment	Mu		k-ft/ft	Wu * L^2 / 8
	ф	0.90	1	Manual - 1: 112 140
	R	0.014		Mu/ phi *b* d^2
required *		0.014		(.85*f'c*b*d)/fy)*(1-sqrt(1- (2*R/.85f'c)
#6 spacing			in^2	(4)-140)(4)
	S	8.00	in	(Ab*12)/ A(required)
	As(min)	0.1296		.0018 (b*h)
	As/ft	0.660		Ab*12/ spacing
Reinford	cement Check	Pass		
		Select Steel Reinfo	rcement C	Check
	Rebar	#6		
	A	0.660	in^2	
	Diameter	0.44		
Spacing	S	8.00	in	
	n	130		
	Total length	1089.2		
Total Width of Span	В	1116	in	
Percent Error		2.5	%	
Max Steel F	Reinforcement	Pass		0%-3% error
		Check and D	rawings	
Moment	Mu	3.62	k-ft	((Wu)(L^2))/8 (simply supported)
	ф	0.90		
Nominal Moment	Mn	4.02	k-ft	Mu/ф
	Z	0.37	ft	=0.9*d
	T	10.94	kip	Mn/Z
Area of Steel Required	As,req	0.18	in^2	
Number of rebars	n	130		
Rebar cross-sectional area	A	0.44	in^2	
Area of Steel	As	57.2	in^2	
Reinforcement Check		PASS		
Minimum Section Width Che	≥ck			
Minimum width	bmin	231	in	From Table 5
Section Width Check		PASS	711	All Assets
Minimum Section Height Ch	eck			
rebar diameter		0.75	in	from Table 5
strirrup diameter		0.375		
minimum clear cover		2.5		- 1
Minimum Clear cover		3.69		hmin=d+db/2+ds+Cc
Minimum Height Check		PASS 5.69	111	mmin-u-ub/2-us-tec
willing neight check		1133		
Ainimum Reinforcement Ch	CUN	35.9143		p/bd
Minimum Reinforcement Ch	0			from Table A.7
Minimum Reinforcement Ch	ρ			HOM Table A.7
	ρ pmin	0.0285		
Reinforcement Check				- Experience of the control of the c
Reinforcement Check Ductility Check	pmin	0.0285 PASS		A Company of the Comp
Reinforcement Check	pmin ECU	0.0285 PASS 0.003	in/in	
Reinforcement Check Ductility Check	pmin ECU a	0.0285 PASS 0.003 0.60	in/in	As*fy/0.85*f'c*b
Reinforcement Check Ductility Check	pmin ECU	0.0285 PASS 0.003 0.60 0.75	in/in in	Fromt Table A.7
Reinforcement Check Ductility Check	pmin ECU a	0.0285 PASS 0.003 0.60	in/in in	
Reinforcement Check Ductility Check	pmin εcu a β	0.0285 PASS 0.003 0.60 0.75 0.80	in/in in	Fromt Table A.7
Reinforcement Check Ductility Check Yield Strain of Steel	pmin  ECU  a β  c  ES	0.0285 PASS 0.003 0.60 0.75 0.80 0.02	in/in in	Fromt Table A.7 a/β
Reinforcement Check Ductility Check	pmin  ECU  a β  c  ES	0.0285 PASS 0.003 0.60 0.75 0.80	in/in in	Fromt Table A.7 a/β
Reinforcement Check Ductility Check Yield Strain of Steel Ductility Check	pmin  εcu a β c εss	0.0285 PASS 0.003 0.60 0.75 0.80 0.02	in/in in in in	Fromt Table A.7 a/β

Table 10: ACI Reinforcements for Straight Overpass Slab



Figure1: Deformation of Right Side of Straight Slab

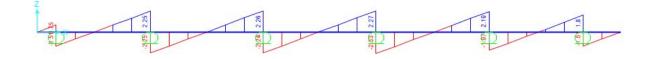


Figure 2: Shear of Right Side of Straight Slab

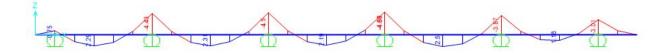


Figure 3: Moment of Right Side of Straight Slab

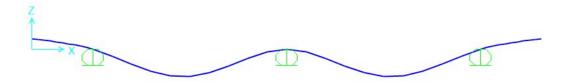


Figure 4:Deflection shape of Curve Slab

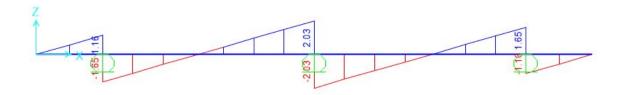


Figure 5: Shear of Curve Slab

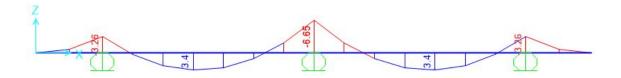


Figure 6: Moment of Curve Slab

### **Design Report - Pedestrian Bridge**

### **General Description:**

The design of the Pedestrian Bridge steel spandrel arch bridge was heavily influenced by the idea of preserving the ecosystem of the San Elijo Lagoon. The steel spandrel arch will span over the lagoon and sit on an abutment on either side of the lagoon. As a design build firm, we believed that a bigger foundation under the bridge was a proper trade off for preserving the ecosystem of the lagoon.

### Materials:

The bridge design is made possible through the use of wide-flange steel beams and braces. For the bridge deck, it consists of steel W shape members that are connected with steel bolts. The slab that sits above the deck is made of concrete pavement.

### **Description of Structural Members:**

The Pedestrian bridge is to be built to provide pedestrians and cyclists with a convenient and safe crossing area connecting Manchester avenue and the Solana Hills trail .The spandrel braced system will be below the bridge deck and the bridge design will be using a fixed arch system. With the two hinged arch system temperature fluctuations will be controlled and shrinkage will be minimized. The fixed arch system creates a very stiff structure which reduces the most amount of deflection in comparison to other arch bridges allowing there to be one singular span. There are no rotations allowed at the foundations which are created by fixing the arch at the abutments. The bridge is 15.5' in width and spans a total of 410.5'.

### **Load Analysis:**

For the design of the pedestrian bridge the calculations shall be in accordance with the AASHTO LRFD guide specifications for the design of pedestrian bridges, the Code of Standard Practice for Steel Buildings and Bridges, and the ACI 318 code. For the analysis of dead loads, live loads, gravity loads, seismic and wind loading, the LRFD methodology will be used to determine the load combinations. With the deck arch design, the bridge will be able to support longer spans for structures due to their ability

to reduce bending moments within the structure while carrying mostly compression loads. The driving failure to be calculated within the structure will be to ensure that the members supporting the arch deck will not buckle under compression forces. To ensure that the arch members supporting the superstructure do not buckle, we have decided to use structural members that have large moments of inertia, as seen in hollow structural sections.

The dead load is the constant loading caused by the weight and density of the materials used composed of the Spandrel Steel arch and the composite steel concrete deck. To find the dead load of a structure. The density of the material needs to be multiplied by the depth of the slab. Now to find the UDL (uniformly distributed load) of the slab, we take the dead load and multiply the dead load by the area of the slab. The pedestrian bridge will have a voided concrete slab of 16 inches. This will allow the bridges to have a longer life cycle since it is right above the lagoon which can lead to erosion .The uniformly distributed load of the other components of the bridge will also be calculated and added to the total dead load of the I beam structures connecting the HSS circular arch beams and the guard rails above the superstructure.

The live load considered is the loading caused by the weight and area, in motion. For pedestrian bridges, the expected pedestrian traffic to be considered will be cyclists and moving pedestrians. In the pedestrian bridge superstructure the only moving force would be the pedestrians. When designing for live load, it is best to consider the worst scenario for a bridge i.e. the most filled the bridge could ever get. For pedestrian bridges the worst casinario that could be considered would be a live load of 150 pounds per square foot. While the pedestrian bridge will not be expecting this type of loading frequently, it will be designed, with an additional factor of safety, to be able to sustain a live load of 150 pounds per square foot.

Job No.	Sheet:		Rev:	
Job Title Sar	n Elijo Lagoon Re	placement Bri	dge	(3)
Subject Pe	destrian Bridge			70
Client	Made By:	Daniel Sandoval	Date:	11/11/2020
Client	Check By:	diracta	Date:	111112020

### Design Data

General

Span: 410.5 ft

Carriageway: Bracing: Steel Surfacin

Concrete

Location San Elijo Lagoon

Type: Pedestran Bridge

### Loading

Unit weight

Steel W12x45 45lb/ft

W12x152 152lb/ft W18x97 97lb/ft W18x211 211lb/ft w21x201 201lb/ft w21x248 248lb/ft w24x250 250lb/ft w27x281 281lb/ft w30x292 292lb/ft w36x330 330lb/ft w40x655 655lb/ft w44x335 335lb/ft

Concrete 150 pcf

Live Loads:

150 psf

	Job No.	Sheet:		Rev:	
	Job Title San Eli	jo Lagoon R	eplacement Br	ridge	
		rian Bridge			
		Made	Daniel		
	Client	By:	Sandoval	Date:	11/11/2020
		Check By:		Date:	
	<u>C</u>	alculation	Sheet		12:
Steel Dead Load					
W12x45	45 lb/ft				
Length	361.2 ft				
	16254	Ibs			
W12x152	152 lb/ft				
Length	1893.6 ft				
	287827.2	lbs			
W18x97	97 lb/ft				
Length	405.2 ft				
	39304.4	lbs			
W21x201	201 lb/ft				
Length	96 ft				
	19296	lbs			
W21x248	248 lb/ft				
Length	1279.5 ft				
	317316	Ibs			
W24x250	250 lb/ft				
Length	708.7 ft				
	177175	lbs			
W27x281	281 lb/ft				
Length	97.5 ft				
	27397.5	Ibs			
W30x292	292 lb/ft				
Length	135.5 ft				
a de Beco	39566	lbs			
W36x330	330 lb/ft				
Length	135.5 ft				
-	44715	lbs			
W40x655	655 lb/ft				
Length	87.5 ft				
100 T 100 P	57312.5	lbs			

	Job No.		She	et:			Rev:		
	Job Title	San Elijo	Lag	oon F	Repla	cement	Bridge		
1	Subject	Pedestria	an Bi	ridge					
			Ma			aniel			
	Client		By:		Sa	ndoval	Date:	1	1/11/2020
			Che By:				Date:		
		Cal	cula	ation	Sh	eet			
W44x335	335	lb/ft							
Length	264.9166								
	88747	.061	lbs						
W18×211	211	lb/ft							
Length	48								
	10128		lbs						
Concrete Dead Load									
Initial Design		-							
Width of deck:	15.5								
Slab Thickness:	4	in.							
Concrete Dead Load	450								
Density of Concrete	150	-							
Top Slab Thickness		in		0.33	IT	6060.75	64.5		
Area of Top Slab	410.5 X	15.5	=			6362.75	11-		
Dead Load Concrete	3817650	lb							
Dead Load Steel	1125038.7	lb							
Total Dead Load	4942688.7	lb							
Effective Span Length	15.5	ft							
UDL=		88.7/15.5	=		31	8883.14	lb/ft		
Live Load									
Loaded Length	410.5	ft							
Loaded Width									
Live Load		psf							
Total Live Load	862050	lb							

Table A: Pedestrian Bridge Design Loads Summary

111111		Slab Informati	on	
Base	b	93	in	
Height	h	5	in	
Effective depth	d	1	in	
Length	L	11.25	ft	
Self Dead Load	WSDL	4.54	K/ft	
Live Load	WL	13.08	K/ft	
		Materials		
Compressive Strength	f'c	4	psi	
Yield Strength	fy	60	ksi	
Type of Concrete		150	pcf	NW
	11	Structural Anal	ysis	
Load Combination	Wu	26.376	k/ft	1.2Wd+1.6Wl
		15.38	k/ft	
Max Moment	Mu	417.2765625	k-ft	((Wu)(L^2))/8
	φ	0.9		
Nominal Moment	17	463.640625		
- DELTH CONTROL OF THE PROPERTY OF THE PARTY	р	0.85	ft	0.9(d)
Area of Steel Required	As,req	79.05		ρ(b)(d)
Checks and Drawings				
einforcement Check				
Number of rebars	n	79		
Rebar cross-sectional				
area	Α	1	in^2	
Area of Steel	As	79	in^2	ОК
neck				
Minimum width	bmin	93	in	OK
neck				
rebar diameter	db	1.128	in	from Table 5
strirrup diameter	ds	0.375	in	
minimum clear cover	Cc	3	in	
Minimum Height	hmin	4.939	in	hmin=d+db/2+ds+Co
			Total Control	ОК
neck				
	ρ	0.753546099		ρ/bd
	ρmin	0.0285		ОК
uctility Check				
Yield Strain of Steel	εcu	0.003	in/in	
	а	14.99051233		As*fy/0.85*f'c*b
	β	0.85		Fromt Table A.7
	C	17.63589686	Annin Control	a/β
	-			

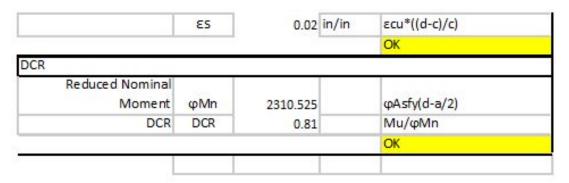


Table B: Concrete Deck Design Summary

### **Bridge SAP 2000 Analysis**

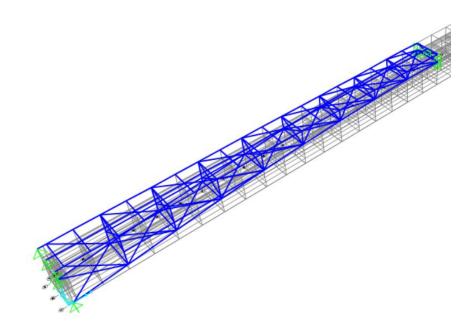


Figure1: SAP2000 Model

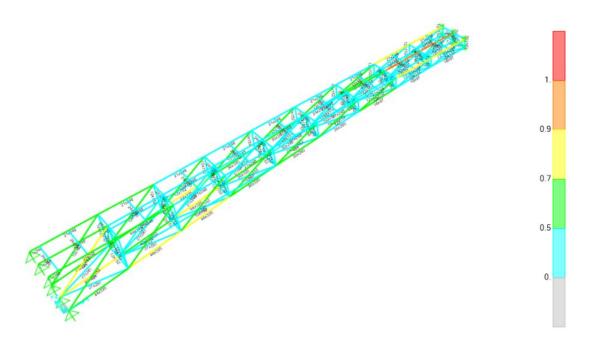


Figure 2: SAP2000 Model

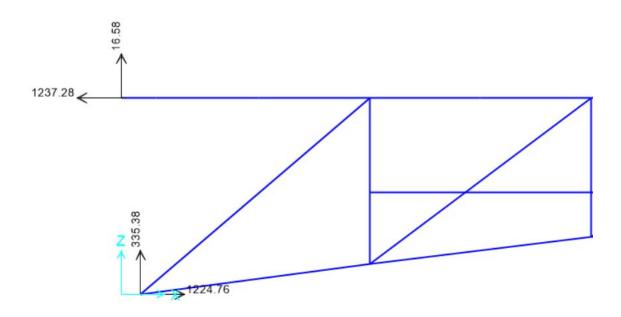


Figure 3 - Reactions Per Bridge Span in kips

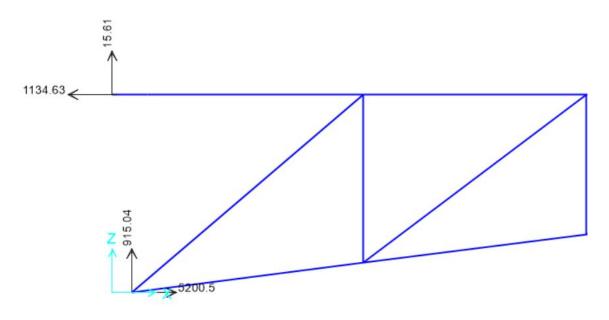


Figure 4 - Reactions Per Bridge Span in kips

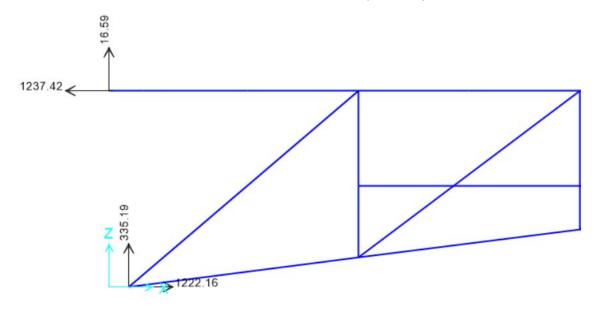


Figure 5 - Reactions Per Bridge Span in kips

### **Design of Abutment:**

The pedestrian bridge will have two abutments on either end of the bridge to sustain the vertical and horizontal loads from the bridge. Due to the nature of the arch bridge, there are lateral thrust forces from the bridge itself that are transferred to the abutments. For this reason, there were parallel wing walls attached to the abutment in order to resist the lateral forces and internal moments. These wing walls were 6 feet long and 4 feet thick. A deep pile foundation was also considered in order to sustain the bearing load on the abutment. There are two points of contact on the abutment one with a bearing pad and the other with a pin like connection to the I beam of the bridge. The overall height of the abutment is 26.5 feet which will allow the deck of the bridge to have enough clearance from the possible 100 year storm event.

### **Design Calculations:**

For the abutment on either end of the pedestrian bridge upholding the deck of the pedestrian bridge. Preliminary dimensions for the reinforced concrete abutment were determined and then calculations were done in order to check the stability of the abutment. Three forms of failure were considered when testing the design of the deep pile abutment. The overturning of the abutment, sliding failure and bearing capacity were considered. These calculations were repeated until a reasonable factor of safety was obtained that complied with CalTrans Standards.

The overturning moment applied to the abutment must be resisted by the opposite moment produced by the vertical and lateral forces. The lateral thrust of the bridge and the pressure of the soil on the abutment were considered when calculating the overturning moment. The overturning moment and the compression lateral force was considered as the overturning moment. The resisting moment was calculated using the total weight of the abutment, the weight of the footing and the applied loading from the pedestrian arch bridge on a single abutment times the respective distance to the abutment toe. To be in accordance with CalTrans standards, the Factor of safety for the overturning of the abutment must be more than 2.0 for shallow foundation. The calculated factor of safety of our abutment design came out to be 0.80 which is less than 1.5. This caused the need for deep pile foundations to resist the overturning moment caused by the lateral thrust of the bridge.

The second method of failure considered was the sliding failure. To comply with CalTrans standards the factor of safety for the abutment must be greater than 1.5. The sliding force is the resultant of the soil pressure acting on the back wall of the abutment. The resitisting sliding force is the self weight of the abutment multiplied by the angle of friction between the soil and the abutment. With the given parameters from the boring logs, the factor of safety for the sliding failure is 2.77 which is greater than the required 1.5.

The third method of failure considered was the bearing capacity failure. The bearing capacity has to be above 3.0 however the bearing capacity calculated for the pedestrian bridge abutment was -0.03 which meant that the foundation would fail due to bearing loads. To counteract this bearing failure piles were added to the foundation to strengthen it. After various rounds of calculations a total of 4 piles (20 inches in diameter and 52 feet deep) are needed on each abutment, 2 rows of 8 piles on each abutment.

### **Abutment Design Calculations Summary**

### **Overturning Calculation on Abutment**

 $\Sigma Wi = 207069.51 \text{ lb/ft}$ 

ΣMo (Resultant Moment) = 4554242.20 lb-ft/ft Overturning Moment: 5677966.20 lb-ft/ft

FS= 0.80 > 1.5 Fail need piles

### **Bearing Capacity**

qu: 358594.50

e: 21

qtoe: 33734.61 governs

gheel: -20375

FS = qu/qtoe = 10.63 > 3 **GOOD** 

### Sliding Calculation of Abutment

Kp: 4.1 Pp: 13,842 D: 6 ft B: 14 ft

FS: 2.77 > 1.5 Ok

### Piles

Bridge load on abutment: 4356 k Total loads on piles: 4356 k # of Piles needed: 20 piles

D = 2 ft

L = 140 ft into bedrock

		Checks For Pedestrain	
D	-1	Geometry	
Base	b	270 in	
Height	h	60 in	
Effective depth	d	58 in	
Length	L	15.5 ft	
Applied Load	P	3609.33 K	
The second and the se		Materials	
Compressive Strength	fc	6 psi	
Yield Strength	fy	60 ksi	
Type of Concrete		150 pcf	Normal Weight or Light Weight
		Structural Analysis	
Moment	Mu	81209.93 k-ft	Cantilever Mu=P(b)
226.31.22.00.12	ф	0.90	5-00
Nominal Moment	Mn	90233.25 k-ft	Mu/ф
	Z	4.35 ft	=0.9*d
	T	20743.28 kip	Mn/Z
Area of Steel Required	As,req	345.72 in^2	
		Checks and Drawings	
Select Steel Reinforcement Check			
Number of rebars	n	114	
Rebar cross-sectional area	A	4 in^2	
Area of Steel	As	456 in^2	
Reinforcement Check		PASS	
Minimum Section Width Check			
Minimum width	bmin	209 in	From Table 5
Section Width Check		PASS	
Minimum Section Height Check		4440000000	
rebar diameter	db	2.257 in	from Table 5
strirrup diameter	ds	0.5 <mark>in</mark>	
minimum clear cover	Cc	2 in	
Minimum Height	hmin	47.00 in	hmin=d+db/2+ds+Cc
Minimum Height Check	- 11	PASS	The first of the control of the cont
Minimum Reinforcement Check			
	ρ	0.0291	ρ/bd
	ρmin	0.0285	from Table A.7
Reinforcement Check		PASS	
Ductility Check			
Yield Strain of Steel	εcu	0.003 in/in	
	a	19.87 in	As*fy/0.85*f'c*b
	β	0.75	Fromt Table A.7
	С	26.49 in	a/β
	εs	0.01 in/in	εcu*((d-c)/c)
Ductility Check		PASS	
DCR			
Reduced Nominal Moment	φMn	98630.1	φAsfy(d-a/2)
DCR	DCR	0.823	Mu/φMn
DCR Check		PASS	DCR<1 (typ values between 0.9 and 1

Table C: ACI Compliance Checks for Pedestrian Bridge Abutment

### Retaining Wall Design:

For the retaining wall design, there will be two different retaining walls varying in height with a max height of 30 feet and a span of over 200 feet long, approximately 240 feet long/. For all the retaining walls we have decided to have them anchored to the soil to provide reinforcement against the soil pressures. There will be nails spaced 5 feet apart from each other with the nails closest to the top and bottom of the wall being 2.5 feet away from the edges of the wall. These anchor nails will be #9 & #10 sized nails at approximately 21 feet long. The nails will be anchored in at 15 degrees from the horizontal. For one wall there will be a total of 288 nails, based off the 240 feet long wall. Fourteen inches of 4000 psi shotcrete will be used to cover the anchored nails and the soil.

In order to determine the total load that the nails are receiving, the total earth pressure load was taken and distributed to the nails based on the assumption that the load is distributed based on the tributary area. A total of 6 nails were considered throughout the wall and their forces were calculated by first obtaining the total load felt on the wall. This would be computed by multiplying the active pressure of the soil by the density of the soil times the Height of the wall squared, times a factor of 0.65.  $F = 0.65K_a\gamma H^2$ . The earth pressure load on the anchored retaining wall has a force diagram of a trapezoid. In order to evaluate the loads sustained by the anchored nails, the load diagram is treated as a rectangle and the missing "triangles" at the outer edges of the trapezoid are subtracted from the overall soil pressure diagram. After this, the load is distributed based off the tributary area of each nail and the loads that they hold.

Due to the limitations of this project, no Factors of safety were able to be computed for the anchored retaining wall however the anchored nails, thickness of shotcrete are an overdesign for the loads the retaining wall is experiencing. The length of the nails were taken from the Anchored retaining wall design manual which suggested preliminary design of the length of the nail shall be 0.7 \* Height of the retaining wall, in our case 30 feet tall.

A square pattern was used when considering the construction of the anchored retaining walls due to the feasibility of the construction of these walls when compared to the triangular distribution of the anchored retaining walls. Although, a bit of strength

is lost, the construction team believed that this method was the better option due to the reduction in schedule and reduction in error during the placement of the nails.

For a 30 ft wall with 6 different anchored nails, spaced 5 feet from each other. At the bottom of the wall will be **R**, the resisting force at the bottom of the retaining wall. These nails are in tension and will be anchored in on the right side of the wall at a 15 degree angle from the horizontal. The force resisted by each nail was computed by assuming that each nail will take the earth pressure load based off the tributary area it holds.

Total Load	22815 psf
Soil Pressure on Retaining Wall	806 pcf
Surcharge	0
Length of Nails	21 ft
Angle of Nail	15 Degrees
Nail Spacing	5ft
Total # of Nails per wall	288
Thickness of Shotcrete	14"
Strength of Shotcrete	4000 psi

Table A: Summary of Typical Retaining Wall Details

The Load distributed on the nails based off T.A.			
Total Earth Pressure	806 pcf		
Tension Nail 1	15649 lbs		
Tension Nail 2	20861 lbs		
Tension Nail 3	20862 lbs		
Tension Nail 4	20863 lbs		
Tension Nail 5	20864 lbs		
Tension Nail 6	15649 lbs		
Resistance at Bottom of Wall	2519 lbs		

Table B: Forces experienced on Nails:

# 15. GreenRoadsCertification



### **Green Roads Plan**

I-5 North Coast Corridor San Elijo Lagoon
Improvement Project

### Main Bridge

Greenroads certification is a ranking system that includes a checklist of categories that include multiple sustainable options that projects can follow in order to deem itself as environmentally safe projects. Greenroad requirements go beyond state and federal rules and regulations. The Greenroads certification is compiled up with mandatory project requirements and subcategories that projects can follow in order to achieve the best score possible. In this proposal, it is listed and will be followed through the requirements needed to receive a bronze certification with the Green Road Rating System Version 2.

Mandatory Project Requirements in Green Roads must be completed for every project that wants to apply for a Greenroads certification. The main aspects of these requirements is to ensure that every project meets the minimum qualifications in order to be considered to receive a Greenroads certification. The main bridge replacement part of this project will have all necessary project requirements for green roads shown in table 1-1 below.

Greenroad Project Requirements	Does Project Satisfy Requirements?
PR-1 Ecological Impact Analysis	Yes
PR-2 Energy & Carbon Footprint	Yes
PR-3 Low Impact Development	Yes
PR-4 Social Impact Analysis	Yes
PR-5 Community Engagement	Yes
PR-6 Lifecycle Cost Analysis	Yes
PR-7 Quality Control	Yes
PR-8 Pollution Prevention	Yes
PR-9 Waste Management	Yes
PR-10 Noise & Glare Control	Yes
PR-11 Utility Conflict Analysis	Yes
PR-12 Asset Management	Yes

Table 1-1: Project requirements needed to obtain Greenroads Certification. Information taken from greenroads.org. Accessed 11/3/20.

	Environment and Water	Pts
EW-1	Preferred Alignment	
-	REQUIRED: Will show that project will not be catastrophically impacted by natural and environmental hazards due to climate change and show that project will stay at acceptable limits if events shall occur per Greenroads spec.  PLAN: We will provide maps and outlines of potential natural hazards that	1
	could occur in the project proximity and plan accordingly to insure that the project will not be affected by costly damages.	
EW-3 -	Habitat Conservation REQUIRED: Will compute the total area disturbed during construction and the total area of project boundaries that use man made hardscape materials and prepare ecological monitoring plans. Will achieve 100% MRA preserved, created, and restored off site per Greenroad spec. PLAN: The construction plan will be carefully prepared to avoid as much environmental impacts as possible.	2
EW-4 -	Land Use Enhancements  REQUIRED: Will compute total impervious area with run off number of 98 before and after construction. Will find total permeable areas and remove hardscape areas and replace them with permeable surfaces and achieve a 5% decrease in impervious area.  PLAN: Areas of this project will be carefully planned and split up by areas in order to achieve the 10% decrease in impervious areas.	2
EW-8 - -	Runoff Flow Control  REQUIRED: Will reduce or eliminate stormwater flow to receiving bodies of water due to construction per Greenroads spec.  PLAN: The construction plan will be carefully prepared to insure that all stormwater flow will have a directional path that leads to larger bodies of water.	1
EW-10	Oil and Contaminant Treatment REQUIRED: Will improve water quality and stormwater runoff beyond basic treatment to reduce oils and nonmetal contaminants per Greenroads spec. PLAN: Construction plans will consider the system of ensuring clean quality water that will be treated to avoid contaminates.	3

Total Points	9

Table 1-2: Environment and Water for Main Bridge Replacement

Construction Activities	PT
CA-1 Environmental Excellence  REQUIRED: Will encourage environmental practices beyond the minimum requirements by selecting a certified contractor or manager with ISO 14001 environmental system per Greenroads spec  PLAN: For our project. Such environmental excellence that can be implemented would be porous pavement and also take in consideration wildlife crossing.	2
<ul> <li>CA-2 Work Zone Health and Safety</li> <li>REQUIRED: Will minimize the amount of safety and health hazards at site during construction by designating a safety officer per Greenroads spec.</li> <li>PLAN: We will have a safety officer/ safety inspector on the site near the bridge, who will have a separate on site safety plan/ manual. This would be implemented from beginning to end of the project, which would require training for workers and subcontractors involved.</li> </ul>	2
CA-3 Quality Process  REQUIRED: Will improve accountability for construction quality by selecting a qualified design builder with ISO 9001 quality management system per Greenroads spec.  PLAN:  Monitoring and measuring equipment calibration records - good  Records of training, skills, experience, and qualifications - good  Product/service requirements review records - good  Record about design and development outputs review - good  Record about design and development input review - good  Records of design and development control  Design and development changes records  Characteristics of product to be produced and service to be provided  Production/service provision change control records  Record of conformity of product/service with acceptance criteria  Record of nonconforming outputs  Monitoring measurement results  Results of the management review  Results of corrective actions	2

### CA-6 Work Zone Water Use REQUIRED: Will encourage responsible use of water during construction and keeping track and recording amount of water use per Greenroads spec. PLAN: For this aspect of the bridge replacement we will not use any water during the construction so recording an amount of water usage is not necessary. **CA-9 Communications and Outreach** REQUIRED: Will encourage engagement with surrounding communities and media per Greenroad spec. PLAN: Regarding the aspect of the project we will have to communicate with the communities that are near the project in order to notify them of potential construction activities that will take place and inform them that it can possibly affect them. The local city will be in coordination with our team to discuss permits and traffic management. CA-10 Fair and Skilled Labor REQUIRED: Will provide fair labor practices and training opportunities for projects but provide necessary records of payroll per Greenroads spec. PLAN: Work tasks will be distributed to subcontractors and will monitor fair practice and training. We will use union based salary if subcontractors are

### **CA-11 Local Economic Development**

willing to do the job.

- REQUIRED: Will stimulate economic development by providing opportunities for local business and local employment per Greenroads spec.
- PLAN: Based on the project development this will boost the economic development by promoting local employment for workers.

Total Points 11

Table 1-3: Construction Activities for Main Bridge Replacement

Materials and Design Requirements	pts
<ul> <li>MD-1 Preservation and Reuse</li> <li>REQUIRED: Will preserve and reuse 70% of existing material within project boundaries and include necessary reports and calculations per Greenroads spec.</li> <li>PLAN: Based on the project we will use recycled asphalt pavement, recycle cement, and recycle steel.</li> </ul>	2
<ul> <li>MD-5 Local Materials</li> <li>REQUIRED: Will make an itemized list of materials and products used and compute the total cost, the percentage of cost being distributed to</li> </ul>	

suppliers. Will achieve 70% of cost sourced locally and prepare necessary maps and records that show materials bought and used are within 50 miles radius per Greenroads spec.  - PLAN: Asphalt, cement, steel, reinforced concrete, and other materials needed for construction will be taken from local sources.  MD-6 Long-Life Design  - REQUIRED: Will design 60% of trafficked areas to meet lifelong criteria per Greenroads spec.  - PLAN: Shows on specification drawing and details the construction techniques, materials utilized on projects that follow the criteria the green road specs.	
Total points	8

Table 1-4: Materials and Design Requirements for for Main Bridge Replacement

Utilities and Controls	Pts
<ul> <li>UC-2 Maintenance &amp; Emergency Access</li> <li>REQUIRED: Will improve the safety and mobility for routine maintenance and emergency vehicles on project site per Greenroads spec. (1 point)</li> <li>PLAN: On site we will hold safety meetings on a monthly basis to improve our safety. Traffic management will be implemented by the safety engineer to coordinate mobility.</li> </ul>	1
<ul> <li>UC-7 Traffic Emissions Reduction</li> <li>REQUIRED: Will reduce operational mobile source emissions and achieve 40% reduction in all air emissions or vehicle miles traveled per Greenroads spec.</li> <li>PLAN: We can reduce traffic emission due to the location of the manufacturing facility for the construction of the prefabrication. The facility is located near the project site speeding up the delivery system.</li> </ul>	2
<ul> <li>UC-8 Travel Time Reduction</li> <li>Computing a reduction travel time in the as-built condition and providing a report of time expected.</li> <li>PLANSince we have a set of precast elements we can efficiently provide a travel system to work for the project. Precast elements include portions of the tower and bridge.</li> </ul>	1
Total Points	4

**Table 1-5: Utilities and Controls for Main Bridge Replacement** 

Access & Livability	Pts
<ul> <li>AL-2 Safety Enhancements</li> <li>REQUIRED: Will plan to reduce existing and potential safety hazards with the use of quantitative safety analysis per Greenroads spec.</li> <li>PLAN: When construction is in process of elevating material/ equipment we will create a zone where pedestrians will not be allowed as an additive source as a factor of safety. Also, secluding areas of drop of delivery of equipment and materials will also serve as a safety factor</li> </ul>	2
<ul> <li>AL-4 Equity and Accessibility</li> <li>REQUIRED: Will select alignment alternatives to minimize adverse social, economic development, community, and cultural impacts to impacted communities per Greenroads spec.</li> <li>PLAN: When the project proceeds with building and construction, plans to minimize the disturbance of local communities such as safety and quality assurance will be implemented.</li> </ul>	1
<ul> <li>AL-5 Active Transportation</li> <li>REQUIRED: Will plan to improve project facilities for pedestrians and cyclists and other active modes of transportation by designating priority or dedicated access per Greenroads spec.</li> <li>PLAN: The project will have a new pedestrian bridge design and built to give cyclists and pedestrians access to move from one end of the bridge to the other. This ensures locals and the community that this project is designating a part that is safe and for their use only.</li> </ul>	1
<ul> <li>AL-8 Culture and Recreation</li> <li>REQUIRED: Will raise awareness of project's culture and recreational resources by providing budget, wayfinding, information, and photo documents per Greenroads spec.</li> <li>PLAN: Once construction is done and the project is finished, signs and photo documents with the project's information and goals will be provided on site.</li> </ul>	1
<ul> <li>AL-10 Scenery and Aesthetics</li> <li>REQUIRED: Will enhance user's visual experience through beautiful views and aesthetic improvements per Greenroads spec.</li> <li>PLAN: The design of our project is unique and aesthetically pleasing bringing in locals and tourists and will urge them to come back to the facility.</li> </ul>	2
Total Points	7

Table 1-6: Access and Liability for Main Bridge Replacement

Creativity & Effort	Pts
<ul> <li>CE-1 Educated Team</li> <li>REQUIRED: Will reward educated project teams and show that project has at least one STP Project Associate per Greenroads spec.</li> <li>PLAN: At the end of the project, awards would be given to people that expressed their excellence in their expertise and made sure every project member was educated in the project and their designs.</li> </ul>	1
<ul> <li>CE-2 Innovative Ideas</li> <li>REQUIRED: Will reward developments of new and innovative ideas associated with design and construction for the project per Greenroad spec.</li> <li>PLAN: At the end of the project, awards would be given to people that expressed great, innovative ideas in design and construction.</li> </ul>	2
<ul> <li>CE-3 Enhanced Performance</li> <li>REQUIRED: Will reward performance achievements and projects that go beyond specs and requirements achieving 3 increments per Greenroads spec.</li> <li>PLAN: At the end of the project, awards would be given to people that expressed extra performance with work that went beyond just the required specifications.</li> </ul>	2
Total Points	5

Table 1-8: Creativity and Effort for Main Bridge Replacement

### **Overpass**

### **Project**

The overpass consists of two bridges that overlap one another to allow transportation to flow freely above and underneath. The Green Road certification is strictly mandating the sustainability of all structures and ensuring the safety of all humans and all of the environment around. In this proposal, the firms, myself and the team, have to list and follow through some requirements to receive a bronze certification with the Green Road Rating System Version 2.

### Requirements

Mandatory Project Requirements in Green Roads must be completed for every project that wants to apply for a Greenroads certification. The main aspects of these requirements is to ensure that every project meets the minimum qualifications in order to be considered to receive a Greenroads certification. The two Overpass bridges will satisfy all the project requirements for green roads shown in table 1-1 below and different sections from each category for Green Road Certification.

Greenroad Project Requirements	Does Project Satisfy Requirements?
PR-1 Ecological Impact Analysis	Yes
PR-2 Energy & Carbon Footprint	Yes
PR-3 Low Impact Development	Yes
PR-4 Social Impact Analysis	Yes
PR-5 Community Engagement	Yes
PR-6 Lifecycle Cost Analysis	Yes
PR-7 Quality Control	Yes
PR-8 Pollution Prevention	Yes
PR-9 Waste Management	Yes
PR-10 Noise & Glare Control	Yes
PR-11 Utility Conflict Analysis	Yes
PR-12 Asset Management	Yes

Table 2-1: Project requirements needed to obtain Greenroads Certification. Information taken from greenroads.org. Accessed 11/3/20.

Environment and Water	PTS
EW-1 : Preferred Alignment	
<ul> <li>REQUIRED: Will show the minimized and avoided loss, degradation, fragmentation, and climate change hazards on the project.</li> <li>PLAN:Ensuring the project does not fall within any farmland or "sensitive" habitat, or within a 100-year floodplan.Look over the Climate Change Vulnerability Assessment to look at any wildfire hazards, extreme weather conditions, and sea level rise hazards.</li> </ul>	2
EW-2: Ecological Connectivity	
<ul> <li>REQUIRE: Will show the reduced habitat fragmentation impacts that would possibly have any improvement on the biodiversity and the different species of plants and animals.</li> <li>PLAN: There will be a complete survey to know the surrounding conditions to consider the current and historical migratory behaviors and evaluate all species impacts, threatened and endangered. This is going to be based on the changed area and perimeter after the completion of the project. After the completion, there will be a net gain of 5% of connective space dedicated for wildlife at ground level.</li> </ul>	2
EW-4 Land Use Enhancements	
<ul> <li>REQUIRE: Will reduce the use of hardscape areas and increase greenspace within the project boundary.</li> <li>PLAN: Hardscape will be used minimally by only providing for traffic, and no additional paths or walkway. The increased greenspace will be implemented on the side of the abutment walls to give a more architectural feel. This will lead to a 10% decrease in impervious areas.</li> </ul>	2
Total Points	6

Table 2-2: Environment and Water for Overpass Bridge

Construction Activities F	Pts	
---------------------------	-----	--

### **CA-2: Workzone Health and Safety**

- REQUIRE: To ensure the safety of the workers and citizens of the city to be protected and minimize the health hazards and dangers during the project construction
- PLAN: There will be a certified prime construction management firm that follows the OHSAS 18001 Health and Safety Management System. There will also be a safety Officer/ Engineer that will oversee the project everyday and take notice of any safety performances that need to be improved weekly. All injuries and illness will be recorded to records.

### **CA-6: Workzone Water Use**

- REQUIRE: To encourage the use of the water resources provided responsible during the duration of the project.
- PLAN: Will create a spreadsheet that records total water use during Project construction. This spreadsheet will keep records of when, where and how much of water was used and reason of usage. This will allow us to calculate the total cost of water used at the end of the project. Lastly, show that 100% of the water sources are non-potable.

### **CA-7: Accelerated Construction**

- REQUIRED: Will implement an organized plan to reduce the time of a project to be delivered, in order to reduce construction congestion
- PLAN: One way we will minimize traffic congestion is to report what is the peak hours
  of traffic and will work before and after those times. By collecting this traffic data, it
  will allow us to organize a schedule before construction begins, reducing the cost by
  10%. This cost will be based on the Federal Highway Administration's Work Zone
  Road User Costs.

### CA-9: Communications and Outreach

- REQUIRED: Will be in constant communications with local communities and media during construction to keep citizens updated for road updates.
- PLAN: We will create a resource for communication and outreach activities for social media to constantly update any changes on lane closures. We will also be providing adequate sides located throughout the construction showing dates that there will be changes in the future plans. All of this will have a monitor of how effective the communication is and how it can improve for the citizens.

### CA-10: Fair and Skilled Labor

- REQUIRED: Will measure the cost of labor OR to account for the total number of full-time equivalent (FTE) hours worked by paid employees

.

3

2

1

2

- PLAN: Will investigate the local construction industry's median wages, benefits, and other allowances paid for all required trades and construction staff on the project and develop a system that will track the hours and productivity of our workforce.

Total Points | 10

Table 2-3: Construction Activities for Overpass Bridge

Materials and Design	Pts
<ul> <li>MD-1: Preservation and Reuse</li> <li>REQUIRED: Will encourage the practice of preserving and resume existing materials during the duration of the construction.</li> <li>PLAN: For the bridges, asphalt pavement, cement, steel, and dirt will be recycled throughout the duration of the project. This can lead to a minimum 60% of the materials used by volume size will be reused or recycled.</li> </ul>	2
<ul> <li>MD-5: Local Materials</li> <li>REQUIRED: Will encourage the use of materials in the surrounding areas to minimize long transportations</li> <li>PLANS: The two overpass bridges are being prefabricated by local concrete companies to reduce any long haul. For asphalt, cement, steel and dirt will all be bought from local companies to minimize any additional cost and shipping/ transport for the materials.</li> </ul>	4
<ul> <li>MD-6: Long- Life Design</li> <li>REQUIRED: To design a long-lasting project to reduce any additional maintenance that is needed for the future.</li> <li>PLAN: We will create a spreadsheet indicating the total trafficked area, the structural condition data, and the lifetime equivalent single axle loads. This will provide a more defined design on the structure that will be approved by the Owner before construction begins. It will then be computed for a 40, 75 and 100 year minimum expected service life for any maintenance that may be needed.</li> </ul>	3
Total Points	9

Table 2-4: Materials and Design for Overpass Bridge

<u>Utilities and Controls</u>	Pts
<ul> <li>UC-2: Maintenance and Emergency Access</li> <li>REQUIRED: To provide an adequate mobility for routine maintenance and emergency activities on the project.</li> <li>PLAN: The bridges are designed to allow additional room on the sides or any last minute maintenance that are needed. They will also be</li> </ul>	1
constructed with appropriate signage designating maintenance and emergency access. A site map will be updated weekly to provide up to date information on traffic flow.	
<ul> <li>UC-6: Lighting and Controls</li> <li>REQUIRED: Utilizing controlled systems and technologies to improve the environmental quality within the surrounding areas of the project.</li> <li>PLAN: The use and installation of fixed lighting systems and controls that result in zero upright above the nadir (BUG Rating UL = 0 and UH = 0), lighting systems mounted at 0 degrees with no tilt, and correlated color temperatures (CCT) approximately moonlight. Also, we may seek to acquire lighting features that have the Fixture Seal of Approval (FSA) by the International Dark-Sky Association. The goal for this section would be to improve the environmental quality by reducing light pollution.</li> </ul>	3
<ul> <li>PLAN: A spreadsheet will be designed to compare the baseline travel times for existing, added to existing projects, and new projects, to compute the total person hours required to travel from one end of a project to the other. During the construction, there will be an increase, so then a construction phase will be designed to show where the expected travel lane will occur at different given times.</li> </ul>	2
Total Points	6

Table 2-5: Utilities and Controls for Overpass Bridge

Access and Livability	Pts
<ul> <li>AL-1: Safety Audit</li> <li>REQUIRED: Will encourage systematic and Transparent evaluation of existing and potential operational safety hazards on the project.</li> <li>PLAN: A formal safety audit will be illustrated with document exchange, several field visits, and recommendations from the project team. This will be completed before construction begins and another during construction.</li> </ul>	2
<ul> <li>AL-4: Equity and Accessibility</li> <li>REQUIRED: Provide a net social benefit, and the universal accessibility to help the project construction and future usage.</li> <li>PLAN- For the socially preferred alignment, values of benefit and cost will be developed to understand the best alternatives that minimize the adverse social, economic development, community and cultural impacts. Large vehicles will be a common occurrence for this project and after the project. The design of both bridges are constructed to provide appropriate spacing and turning for large vehicles. All be able to be seen in the structural loading calculations, geometric calculations and design, and the stop structural improvements.</li> </ul>	2
<ul> <li>AL-8: Culture and Recreation</li> <li>REQUIRED: Understanding the local cultural and recreational resources that can be implemented into the design and construction</li> <li>PLAN: This project is located in Southern California in the Coastal line, meaning that the design will be designed more for a flow and represent the ocean waves onto the bridge. The budget will be dedicated to enhance significant cultural, community and recreational sites to help with funding ideas. Resources about the local community will be used to compute the actual true value designated by the project upon completion.</li> </ul>	2
<ul> <li>AL-10: Scenery and Aesthetics</li> <li>REQUIRED: Improve the visual experience throughout the project completion by implementing aesthetic improvements.</li> <li>PLAN: The bridges will be constructed with a nice flow of a curve while turning. On the edges of the bridge will be parapets that will give a more</li> </ul>	2

finished work, than a simple railing. The abutments walls will as well be provided with a scenery form that will represent the beaches of that local community.

Total Points 8

Table 2-6: Access and Livability for Overpass Bridge

Creativity & Effort	Pt
<ul> <li>CE-1 Educated Team</li> <li>REQUIRED: Will reward educated project teams and show that project has at least one STP Project Associate per Greenroads spec. (1 point)</li> <li>PLAN: At the end of the project, awards would be given to people that expressed their excellence in their expertise and made sure every project member was educated in the project and their designs.</li> </ul>	1
<ul> <li>CE-2 Innovative Ideas</li> <li>REQUIRED: Will reward developments of new and innovative ideas associated with design and construction for the project per Greenroad spec.</li> <li>PLAN: At the end of the project, awards would be given to people that expressed great, innovative ideas in design and construction.</li> </ul>	2
CE-3 Enhanced Performance - REQUIRED: Will reward performance achievements and projects that go	
<ul> <li>beyond specs and requirements achieving 3 increments per Greenroads spec.</li> <li>PLAN: The performance of each individual on the project is working to achieve the Greenroad specification to provide a better construction and design of the whole project.</li> </ul>	
Total Points	5

Table 2-7: Creativity and Effort for Overpass Bridge

### Conclusion

Both Projects are dedicated to make the most of this project to receive a bronze certification with Green roads, and may not even stop there. For right now, the main bridge provided a report of 45 points that are due to increase. For the Overpass Bridges, it provided a report of 46 points. There are many opportunities to better the design and construction to better the life span of the structures and the environment around. All of this will be done by looking at the materials, the land that it will use, and the whole transportation around to construct this great main bridge replacement and overpass bridges with connection to the other structures.

## 16. Bidders Questionnaire

### Appendix A



### **Safety Plan**

# **Workplace Safety Program**

#### Introduction

The enclosed workplace safety program for BB Incorporated is to comply with taking measurements of safety within the site of the Project. This workplace safety plan is to ensure a safe free workplace within the environment. These standards should be sent to the jobsite on request.

The safety program is intended to serve as a basis for the employer safety program. The safety program consists of several essential elements/sections:

- 1. Management's commitment and involvement
- 2. Safety committee operation
- 3. Provisions for safety and health training
- 4. Safety checklist
- 5. Preventive Maintenance
- 6. First aid plan
- 7. Accident investigations
- 8. Recordkeeping of injuries
- 9. Job specific safety rules and procedures

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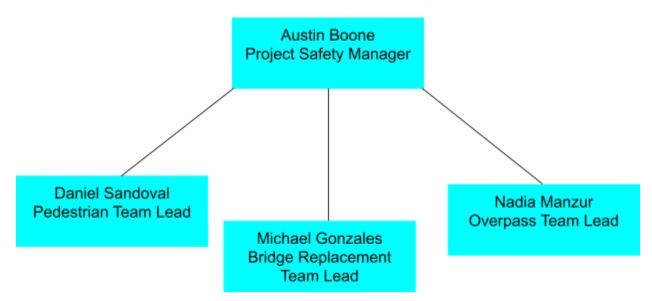
# 1. Management's Commitment and Involvement

The management at BB Incorporated will be committed to preserving life and wellness on this project with the utmost intensity. Some of the main values and themes we will adhere to throughout this planning and execution can be found below:

- Implementing a safety procedure of the type of work
- ensuring that the incidents that happen on site are being reported.
- Takes in consideration of employee opinions and also taking them in consideration when it comes to decision
- Providing a safe and effective tool for support to achieve the desired outcome

# 2. Safety Committee

This project will have a safety committee dedicated to ensuring the well-being and prosperity of everyone on site. We at BB Incorporated believe that everyone returning safely to their families is of highest priority. Because of this, the following organization of people will be developed to uphold the safety of the project:



Each team lead will work with the safety manager to ensure safety for their specific scope. The safety manager oversees safety efforts for the entire project. Meetings will be held monthly and meeting minutes will be well-documented and

# 3. Safety and Health Training

#### 3-1 Safety and Health Orientation

Before starting work on site, every new employee and existing employee are required to undergo a safety and health orientation. Upon attending the Orientation the employees will be informed about the different types of job training, and retraining of employees. Upon completion of the orientation the employee will be given a copy of

the safety rules to use as a reference and also would be given a safety plan pertaining

to their job.

All job training should be documented and recorded too comply with company

safety and health regulation

3-2 Job specific training

• Supervisor training employee and teaching the employee how to perform their

task

Provide online course on job training to illustrate what is acceptable on site and

what is not

• Show new and current employees the use of equipment and also vice versa the

use of new equipment

Supervisor should shadow the new employees to get hands-on experience and

correct the employees as needed when not doing a task correctly.

3-3 Retraining Employees

All employees will undergo recurring training on a monthly basis when the workspace

safety manual gets updated or when new equipment is being brought in on the site.

3-4 Safety training documentation SAMPLE

Employee: Jerry Sienfeld

Date: 10/14/2020

6

Supervisor: Peter Parker	
Trainer: Clark Kent	
Rules and Regulation Reviewed	<u>Date</u>
General review of old and new safety plans	
General Maintenance	
First Aid	
PPE Reviewed	
Machinery Certification	
Equipment Used	
Sanitation/Health	
All categories have been reviewed with the employee.  Supervisor name, Printed:  Signature:	
I have been advise of all safety and health regulation best of my ability Employee name and printed: Signature:	s and will adhere to them to the

# 4. Safety Inspection Checklist

# **Protective Equipment**

Item	yes	no	n/a
Visibility Vest Present on site?			
Hearing protection equipment Present?			
Are workboots Present on site?			
Are safety Glasses present on site?			

# **Fall Protection**

Item	yes	no	n/a
Are parameter edges secured for any falling debris?			
Are construction tools tethered?			
Are workers safely properly tied off			
Are the exterior area working area netted?			
Are their warning			

signs in dangerous areas?		
Is there a 100% fall protection?		

# 5. Preventive Maintenance

For small machinery equipment used on site, the BB incorporate would make sure that regular maintenance is provided frequently to insure the safety of the workers and also would make sure that the small machinery at risk of malfunctioning does not create a hazard danger for the employees

For the purpose of this project, it will be best to use the usage-based maintenance. This would be a good preventive maintenance technique to use for this project since we would have to use heavy equipment for a long period of time. Hence, by using this technique BB incorporate would be keeping track of the heavy machinery that are used frequently.

## 6. First-Aid Plan

The first aid plan will be reviewed periodically to determine if it continues to address the needs of the workplace. The first-aid plan will be adjusted based on the changes on the workplace safety and the health hazards, worksite locations, and worker schedules.

## **6-1 Emergency Personnel Contact Information**

Contact information	Phone Numbers						
Safety Coordinator	Austin Boone Cell						
First-Aid Responders	Call 911 in case of emergency						
Police	San Diego County Sheriff's Department						
Fire department	San Diego FD						
Hospital/Medical clinic							

#### 6-2 Medical Services

For the medical services that would be provided by BB incorporated we would have two options. The first option would be emergency services being provided on site. The second option would be calling the nearest hospital/medical clinic near our jobsite.

## Option 1

- Will provide medical attention in case of injury or illness to employees. The proper equipment for prompt transportation of the injured person to a medical clinic or hospital.

## Option 2

- The nearest medical treatment facility to the worksite will respond to all the emergencies or injuries that have been sustained by the employee. A contact sheet with several telephone numbers have been created in case 911 does not happen to work. BB incorporate will post telephone numbers in conspicuous locations on site.

### 6-3 First-Aid Training

All employees will receive First-Aid and CPR training through BB Incorporated. Upon completion of the training, the employee will be able to administer first aid on site.

#### 6-4 First-Aid Instruction

#### Wounds:

#### Minor Wounds:

- Clean the wound with water and soap to avoid infection
- When handling the wound, it should be taken care of with a clean cloth

#### Major Wounds:

- For major wound, it is best to apply pressure to the wound to stop bleeding with a clean cloth
- Designate someone SPECIFICALLY to call 911

• Should apply pressure to wound until emergency personnel arrives on scene

#### Burns:

# Thermal (Heat)

• Rinse the burn area by scrubbing it, and immerse in cold water

# Eye Injury

# Small particles:

- If small particles gets in the eye, do not rub your eyes
- Can rinse eye with water till particles are out of the eye

# Large or Stuck Particles:

- If a particle is stuck on your eye, do not attempt to remove it
- Cover injured eye with a bandage

# 7. Accident Reporting

## 7-1 Accident Investigation Procedures

In the event of an accident, however minor, an investigation will be conducted to learn the following:

- The who, what, where, when
- The cause of the accident
- Who is responsible
- How we can learn to prevent the accident in the future

OSHA requires reporting within 8 hours for accidents involving fatalities or the hospitalization of 3 or more employees

OSHA CENTRAL TELEPHONE NUMBER: 1-800-321-6742

## 7-2 Sample Accident Report

Report No.:
Company:
Address:
1. Name of injured: S.S. No.:
2. Sex: ☐ M☐ F Age: Date of accident:
3. Time of accident:a.mp.m. Day of accident:
4. Employee's job title:
5. Length of experience on job:years:months
6. Address of location where the accident occurred:
7. Nature of injury, injury type, and part of the body affected:
B. Describe the accident and how it occurred:
9. Cause of the accident:
10. Was personal protective equipment required? □yes □no
Was it provided? □yes □no
Was it being used? □yes □no

If "no," explain:		
Was it being used as trained by a supervisor or designated trainer?	$\square$ yes	□no
If "no," explain:		
11. Witness(es):		
12. Was safety training provided to the injured? □yes □no If "no," explain:		
13. Interim corrective actions taken to prevent recurrence:		
14. Permanent corrective action recommended to prevent recurrence:		
15. Date of report: , 20		
Prepared by:		
Supervisor (Signature): Date:		
16. Status and follow-up action taken by safety coordinator:		
Safety Coordinator (Signature) Date:		

# 8. Recordkeeping

The Project Safety Manager will be responsible for maintaining and filing all safety and accident reports with a backlog of a 5-year minimum. Records kept will include

- Investigation reports
- Worker's compensation claims
- Summary of occupational injuries
- All other forms required by OSHA: 300, 300A, 301, etc.

# 9. Daily Safety Rules and Policies

#### 9-1 Field Employees:

- 1) Use caution signage for slippery or hazardous areas
- 2) Do not leave or store items in paths of travel
- 3) Use appropriate tools and products for field work
- 4) Always lift with the legs and never with the back
- 5) Always test an object's weight before exerting
- 6) Team-lift when necessary to avoid strains
- 7) Face the load directly when lifting
- 8) Set down objects in the same manner they are picked up
- 9) Use ladders that are in good working order
- 10) Always follow the instructions printed on the ladder
- 11) Do not "walk" ladders by rocking them
- 12) Be cognizant of heavy equipment in our area
- 13) Check behind you constantly when operating in known paths of travel

#### 9-2 Office Employees

- 1) Leave hallways and passages clear of chairs, trash cans, etc.
- 2) Do not throw matches or other burning materials directly into trash cans without first wetting them
- 3) Carry and store sharp objects such as pencils and scissors with points facing down

- 4) Obey all posted signage regarding safety in the office
- 5) Do not use electrical appliances with frayed cords or ones that are in otherwise non-working order
- 6) Do not overload electrical circuits by using too many appliances at once.

#### RISK MANAGEMENT PLAN REV.1 - SAN ELIJO LAGOON

			Risk Response Plan							
#	Status	Risk Category	Risk Event	Cause	Effect	Probability	Impact	Risk Matrix	Response Strategy	Response Actions
1	Active	Construction	Construction confilict between pedestrian and main bridge	Working in close proximity and time between the two scopes	Project delay, schedule re- work, logistical issues	Medium	Medium	VH H H X X X VL L M H VH Impact	Avoid	Extensive planning and forethought into the locgistics of phasing, equipment, assembly, safety, etc.
2	Active	Environment	Pedestrian Bridge Flood	100-year Storm Event	Excessive wear, possible structural damage	Low	High	VH A A A A A A A A A A A A A A A A A A A	Mitigate	Monitor Storm and surf activity closely,
4	Active	External	Project not fully funded	budget constraints - allocation in doubt or subject to change	Project delayed	High	Very High	All Med L VL L M H VH Impact	Mitigate	Project may be divided into 2 or 3 phases with options in the contract
5	Active	Design	Inaccurate Cost Estimate	Unit pricing affected by rick excavation and disposal issues	EE is underestimated	Medium	Medium	VH H XIII QRQ L VL L M H VH Impact	Mitigate	Increased unit price to match rock excavation price
6	Active	Construction	Unidentified Utility Impacts	Unidentified Utilities	Project Cost Increases	Low	Low	VH H H WI M H VH Impact	Transfer	Contingency plan. Contractor is responsible for coordination
7	Active	External	Permit Delays	Permits expire, permits or agency actions are delayed or take longer than expected	Fines, penalties, project delays	Medium	Low	VH H X X VL L M H VH	Transfer	Consultant responsible for coordinating permits and identifying permit requirements
8	Active	Construction	Differing site conditions	Unexpected geotech issues, natural or man-made obstructions	Increased project costs	Medium	Medium	VH H H X X X VL L M H VH Impact	Mitigate	Thorough geotechnical investigations performed
9	Active	Project Management	Marketing Opportunities	Public esposure of a high- profile FLH project	Improves viability of FLH	High	High	VH H X X H H VH Impact	Accept	Future marketing of this successful project by the division
10	Active	Safe	Working in elevated heights	On site injuries	gal process, insurance, loss of lif	Medium	High	M H X W W W W W W W W W W W W W W W W W W	Mitigate	Extensive planning and forethought into the assembly of overpass and develop a strict safety protocal to aviod accidents

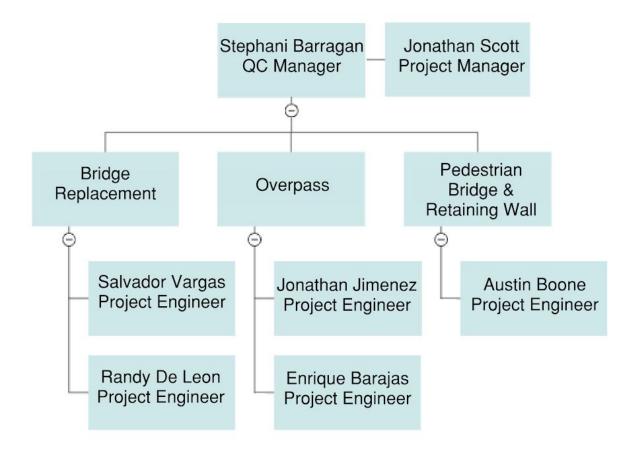
- 1		l t	ty							П		Imp	act			1
	1 34	o de la companya de l	Project Management	Causing major impacts to traffic	Building near/around I-5 freeway and city streets	Heavy traffic congestion, pressure from public		Medium	Low	Probability A A	1		( H	VH	Mitigate	Plan an effecient schedule and traffic management plan. Increase communication to public to avoid public issues
	2 sito	o de la companya de l	Construction	Noise control	Construction near residental housing	Public issues with local residents, cause delays		Medium	Low	Probability A A	1	X L !	M H	VH	Mitigate	Commnicate with public and surrounding communities. Attempt to stay on schedule and stay within agreed operating hours.
	3 3	Sa	afe ty	Covid-19	One case can turn into multiple. Put workers/staff in danger	May cause job delay. Potential for heavy media attention. Health of personnel at risk.	N	Medium	Very High	Probability A 1 A	1			VH	Mitigate	Social distancing, required masks, required gloves, and other safety procedures. Will supply site with cleaning supplies and hand sanitizer.
	4 4	original designation of	Construction							Probability A A		L I	M H	VH	Mitigate	

# **Quality Management Protocol**





# **Organizational Chart**





This "Quality Management Protocol" (QMP) documents the necessary tasks to achieve the specific goals of the **San Elijo Lagoon Project.** 

Success of this project depends on the best people fully understanding the goals as defined by the contract and working at their highest level in concert to achieve those goals. For purposes of this document the project goals are defined as "critical success factors". Communicating these factors throughout every person working on the project will assure these success factors are built into every detail of the project. Communication is the cornerstone to the success of this project.

The "Protocol" consists of two elements, "The Quality Assurance Plan (QAP)" and the "Quality Control Plan (QCP)".

The QAP is the planning and **preparation** required to assure every critical success factor is accomplished. The QCP defines the procedures to **measure**, **track and build** the critical success factors into the physical project.

#### 1.1 Quality Assurance

**Quality Assurance** Is the preparation to achieve Quality. Its fundamental purpose is to define the people, methods and tools to achieve excellence and prevent any compromises from jeopardizing the achievement of any of the "Critical Success Factors". The key components to our "Quality Assurance Plan" are:

- 1. A MUTUAL UNDERSTANDING OF QUALITY EXPECTATIONS AND CRITICAL SUCCESS FACTORS
- 2. ASSEMBLING and MAINTAINING the BEST TEAM
- 3. UNDERSTANDING THE UNIOUENESS OF THE PROJECT
- 4. COMMUNICATING: THE ALIGNMENT OF ALL TEAM MEMBERS
- 5. CLEAR DECISION MAKING PROTOCOL
- 6. BUILD BEFORE WE BUILD
- 7. UNDERSTANDING THE MATERIALS/EQUIPMENT
- 8. ADOPTING BEST PROCEDURES
- 9. SUBMITTALS AND MOCK-UPS
- 1. A MUTUAL UNDERSTANDING OF QUALITY EXPECTATIONS: BB Incorporated will employ the following processes to establish a mutual knowledge of the critical success factors by all who work on the project for the full duration of the project.



#### A. CONTINUOUS DESIGN CONSTRUCTABILITY REVIEW

**BB Incorporated** along with all subcontractors will conduct a **Continuous Design-Constructability Review**. Our approach utilizes checklists to assist in the creation of consistent, thorough and coordinated construction documents throughout the project. This review of the contract documentation is done as preparation to the "Quality Workshops" and serves as a communicative tool to verify costs, quality and schedule are consistent with the expectations of the client and the entire project team.

- 1. People **BB Incorporated** and Subcontractors Construction Project Manager(s), and Project Engineers participate in the continuous review process on an as needed basis. Open communication with clients and design during the project is required for the continuous review process. All found conflicts, omissions or clarifications must be shared regularly. "Quality Workshops" to examine and solve the above will be scheduled regularly (once a week is recommended) **with decision makers** in the workshops. Attendees for these workshops will be defined by the specific topics needing resolution.
- 2. All suggestions, corrections and concerns are communicated during the Quality Workshops and are fully documented.
- 3. All items not addressed or resolved during the Quality Workshops must be scheduled for resolution prior to the start of all affected construction. Items not able to be reconciled during the workshop must be agreed upon by all parties and must be reviewed for impact on schedule, costs and quality.

#### **B. QUALITY WORKSHOPS**

**BB Incorporated** (and subcontractors and consultants as needed) will attend the **Quality Workshops**. The first Quality Workshop has the singular goal to come to a full, and mutual understanding by all parties of the "Critical Success Factors" which define this project. Throughout a project the "Critical Success Factors" will not change but how they are achieved may evolve. It is the intent of the Quality Kick-off Workshop to confirm the understanding of all expectations of all parties involved in the project. It is the intent of the ongoing Quality Workshops to address and reconcile all possible obstacles to the successful completion of the project. BB Incorporated's designated quality lead will coordinate the workshops with all parties. The agenda for the Quality Workshops includes:

- 1. A thorough understanding of all of the lines of communication
- 2. Agreement to all standards of construction affiliations and references used on the job site.
- 3. An understanding by all of any missing information or inconsistencies within the RFP, the Response to the RFP, the instructions to bidders and the contract documents (schedule may affect the contract documents)
- 4. Discussion of all possible mitigating solutions
- 5. Discussion of all Design Review Checklist items
  - 1. Schedule corrections and future inclusions
  - 2. Anticipated bulletin dates



- 6. Discussion of all mock-ups defined in contract documents AND others which may be recommended
- 7. Discussion of all First-in-Place procedures and their associated meetings
- 8. Discussion of all Inspections and Testing procedures
- 9. Decide, as a team, the criteria to be met to achieve all quality requirements.
- 10. Quality expectations.
- 11. Acceptance criteria and accountability.
- 12. Verification appropriate shop drawings and material/equipment submittals have been submitted and approved prior to installation of related work and verification of receipt of approved factory test results when required.
- 13. Review of the Inspection and Testing plan and ensure provisions have been made to provide the required quality inspections and tests. Identification of the inspections and tests to be performed, the frequency, where required, acceptance criteria, and who is to perform the inspections and tests. Include all Special Inspections required by all governing agencies
- 14. Discussion of construction methods and tolerances, workmanship standards, and the approach to be used to provide quality construction by planning ahead and identifying potential issues.

## 2. ASSEMBLE and MAINTAIN the BEST TEAM

#### A. PEOPLE

BB Incorporated realizes an enthusiastic attitude, strong knowledge and excellent skills are essential to the successful and full execution of the contract. With an emphasis on quality communication and full accountability by all team members, we will only select people from our organization and will only accept people from consultants and subcontractors which possess the following criteria:

- 1. Excellent communication skills
- 2. Excellent fit with the entire Project Team
- 3. Proven Experience
- 4. Possess enthusiasm and interest in working on this project with an understanding of all factors including but not limited to the schedule and budget which define the success of the project.
- 5. Possess all license and certification requirements as necessary for their defined tasks
- 6. Eager to participate in ongoing education and training to stay current with industry standards
- 7. Recognize the most important quality of a successful team is its ability to communicate within and with others
- 8. Realize attitude will get everything done well

Each trade partner will designate a key individual as their "Quality Lead". Satisfying all criteria of 1-8 above this individual is responsible for all communication regarding all aspects discussed in this Quality Management Protocol. Specifically and as a minimum this



person is responsible for the preparation of all Quality submittals, reports and documentation. Further, this person will attend and actively participate in all Quality meetings, reviews, inspections, walks and all other activities as necessary or as requested by BB Incorporated to satisfy the expectations of this Quality Management Protocol.

**3. UNDERSTAND THE UNIQUENESS OF THE SPECIFIC JOB:** We understand the uniqueness of the **Project Name** project. The following constraints will be considered. This may not be a comprehensive list and other constraints must be considered. Time should be allotted within the first Quality Workshop to discuss all potential project constraints.

#### A. CONSTRUCTION CONSTRAINTS

- 1. CONSTRUCTION REQUIREMENTS
  - a. Allowable working hours
  - b. Traffic requirements
  - c. Sound requirements
  - d. Lighting requirements
  - e. Public property protection requirements
  - f. Adjacent property rights and regulations
  - g. Utility connections and disconnections
  - h. Pedestrian protection
  - i. Demolition restrictions
  - i. Dust control
  - k. Fencing requirements

#### 4. COMMUNICATION: THE ALIGNMENT OF ALL TEAM MEMBERS

- **A. INITIAL COMMUNICATION:** A Subcontractor Preconstruction Meeting will be conducted with every entity joining the team as part of the orientation to the project. In addition to a thorough review of the critical success factors, our team will review with every sub-contractor and consultant the following:
  - 1. Required "Subcontractor Quality Control Plans" (These will be collected from each subcontractor and consultant. Hardcopies will be assembled into a jobsite binder and a digital copy will be recorded.
  - 2. Quality expectations
  - 3. Project schedule (including submittals and procurement schedules)
  - 4. Acceptance criteria and accountability of all work
  - 5. All applicable specifications
  - 6. Verification of appropriate shop drawings
  - 7. Verification of sample submittals
  - 8. Verification of receipt of approved factory test results when required
  - 9. Review of the Inspection and Testing plan and ensure all required quality inspections and tests have been documented. Identify the inspections and tests that are to be performed,



- the frequency, where required, acceptance criteria, and who is to perform them. Include all Special Inspections and all governing agency inspections
- 10. Examination of the work area to ensure that the required preliminary work has been completed
- 11. Examination of the required materials
- 12. Verification of appropriate material storage, length of storage, temperature and humidity regulations.
- 13. Verification that required installation equipment is present and appropriate
- 14. Discussion of applicable construction methods and tolerances, workmanship standards, and the approach to execute quality construction
- 15. Coordination with all other work
- 16. Review of all mockups already completed and approved for quality expectations. Meeting minutes will be taken and distributed to the project team.
- B. CONTINUOUS COMMUNICATIONS: Our success on our projects is rooted in communication. Continual communication ensures all pertinent information, which could affect any aspect of the job, is thoroughly discussed with all appropriate parties. BB Incorporated and appropriate subcontractors/consultants will attend and fully participate in the Quality Workshops and use these as a communicative tool to further collaborate and to thoroughly foresee potential problems, fully vet problems which do arise and fully explore and execute all needed solutions (see Quality Control). Should a question arise which needs attention by an individual or group not represented in the Quality Workshop, it will be communicated through BB Incorporated (Quality Lead or Project Manager), from there to appropriate decision making entities. It is essential all threats to the successful completion of any of the Critical Success Factors be surfaced as early as possible to appropriate parties for resolution. AS A MINIMUM THE FOLLOWING COMMUNICATION TOOLS WILL BE IMPLEMENTED THROUGHOUT THE PROJECT:
  - **1. BB Incorporated Conducted Daily Huddle:** This huddle will include various subcontractors depending upon work being performed. This meeting has the following objectives:
    - **a.** Preparation and organization of work being done specific to the day
    - **b.** Discussion of communication needs surrounding the daily work
    - **c.** Assurance all coordination has been done to perform the work
    - **d.** Discussion and preparation of work coming up in the 3-week look ahead
    - e. Identification of weekend work (tasks, personnel, reg. inspections, etc.)
    - **f.** Review the digitized model (using BIM 360) with the purpose of:
      - 1. Identifying construction clashes
      - 2. Identifying better practices in construction
      - 3. Resolving clashes
    - **g.** Lessons learned (as needed)



- **2. BB Incorporated Internal Weekly Staff Meeting:** This weekly meeting includes a discussion of Lessons Learned AND a job walk by the entire office staff (with the possible exception of administrative personnel). Observations of site conditions MUST be photographed as incurred. A review and planning for the 3-week look-ahead schedule must be discussed
- **3. Quality Workshops:** Attended by the Owner, BB Incorporated and appropriate consultants and subcontractors as needed, this meeting is to assure communication is shared among all team members in a timely manner. As a minimum, this meeting will review Schedule, costs and outstanding issues as pertains to activities under current consideration for testing, inspections procurement, storage, installation or fabrication on the project. It further provides the vehicle for team members to communicate in person any and all concerns regarding the project.
- 4. Special Communication Regarding Weekend or Extra Hour Work

The project schedule shows may require weekends and/or extra hour work. BB Incorporated will put in place a number of protocols to assure quality control is continuous through these times. This protocol includes:

- a. BB Incorporated submittal of a phone tree for use for each specific weekend. .
- b. All subcontractors must submit a list of activities being performed on the weekend to **BB Incorporated** by Thursday morning at 8:00am immediately prior to the weekend to be worked.
- c. All subcontractors must submit a prioritized phone tree of contacts to be called on weekends should questions arise.
- 5. All subcontractors must have at least one Project Manager or one foreman laborer working on the Friday prior to the weekend to be worked.

# 6. Special Communication Regarding Approvals and Documentation of Changes in Scope

The project process includes RFIs and Submittals to align work with project intentions throughout the construction process. In some instances Engineering Judgments (EJs) will be required and associated change orders will ensue. In cases where an interpretation of any kind is required by the Fire Marshall, the following protocol must be followed:

- a. RFI prepared in collaboration with the Fire Marshall (with suggested solution as discussed with team)
- b. RFI formally submitted to all parties (Fire Marshall, design team, construction management team, appropriate affected subcontractors, etc.)
  - i. With approval of Fire Marshall
  - ii. Affect on schedule
  - iii. Affect on budget
  - iv. All affected trades



- c. RFI incorporated in Change Order
- d. Change Order is approved
- **5. CLEAR DECISION MAKING: BB Incorporated** considers the full project schedule as an aggressive schedule. Weekends throughout the project may be necessary to be worked. Decisions must be made in a timely manner WITHOUT sacrificing quality. A clear, concise decision making tree must be established to assure appropriate, decision-making people attend meetings (effective meeting control) and decisions which must be made in "off" hours can be made.

# A. Typical Decision Making Tree

- 1. Not included in contract documents and considered means and methods
  - a. Field made decisions overseen by **BB Incorporated**
  - b. Entered into Project Managers' "Daily Log"
- 2. Not included in contract documents and not means and methods
  - a. Subcontractor raises issue to Project Manager, writes potential R.F.I.
    - 1. Collaborate with others to provide possible resolution to include in R.F.I.
  - b. P.E. with Project Manager reviews R.F.I. for:
    - 1. Appropriateness
    - 2. Other trades which may be involved
    - 3. Possible effect on schedule
    - 4. Possible effect on budget
    - 5. Begins Change Order process (as needed)
    - 6. Further detail needed for job specificity
- 3. Issues affecting change in design, change in budget, change in schedule
  - A. RFI issued (includes suggested resolution through collaboration)
  - B. Raised in Quality Workshop for appropriate action (future change order?)
  - C. Issues not able to be resolved in Quality Workshop advanced to appropriate decision maker

#### **B.** Weekend Decision Making Tree

- 1. Issues not included in contract documents and considered means and methods
  - a. Field made decisions overseen by **BB Incorporated** Project Manager/Foreman
  - b. Entered into Project Managers "Daily Log"
- 2. Not included in contract documents and **not** means and methods
  - a. Subcontractor raises issue to Project Manager
    - i. **BB Incorporated** Project Manager or Foreman reviews issue:
      - a. In contract documents? (is it really an issue? If yes...)
        - a. Other trades which may be involved
        - b. Possible effect on schedule
        - c. Possible effect on budget
        - d. References Weekend Work Communication Tree



- i. **BB Incorporated** Phone tree to be identified by previous Thursday
- e. Project Manager confirms appropriate action with involve parties
- 3. Issues affecting change in design, change in budget, change in schedule
  - D. Project Manager/Foreman assesses urgency of situation
    - i. If situation prevents further weekend work
      - 1. Uses pre-establish Weekend Work Communication Tree

Anticipation: Teaming the experience of the Owner and the entire team with BB Incorporated provides the best possibility for anticipation of potential threats to the budget, the schedule or to the success of any of the critical success factors. The Quality Workshops are essential to the collaborative resolution of inevitable, unanticipated findings and events indicative of every job site. The BB Incorporated team will minimize these to every extent possible. Listed below are elements of the construction process we review thoroughly prior to starting the job or any task on the job. We review these as appropriate in the 3 week look-ahead schedule and in our daily huddles. We fully expect and will demand every subcontractor review the following prior to all quality workshops.

- 1. Industry standards
- 2. Specific requirements of the job in contrast to industry standards (more demanding tolerances, security, environment, schedule, costs, etc. may all have specific needs in respect to the project.
- 3. BIM Subcontractors weekly meeting agenda
- 4. All Contract documents (including submittals, RFIs, bulletins, etc.)
- 5. Posted drawings and their associated and cumulative impact on the entire project
- **6.** Job Hazard Analysis checklist
- 8. UNDERSTAND THE MATERIALS/EQUIPMENT: Understanding the materials and their use is essential to their installation. We look closely at all materials used on the job and consider when they will be needed, the preservation of their quality both prior to and on the jobsite: transportation, storage, installation and in-place protection. A thorough understanding of all of these aspects is required to satisfy schedule, budget and quality. We require our subcontractors to be committed to UNDERSTANDING THE MATERIALS being used on the project.
- 1. **PROCUREMENT:** Within the overall schedule and specifically with respect to the 3-week look ahead schedule, BB Incorporated and all subcontractors must track a number of factors which assure the delivery of the correct materials at the appropriate time:
  - a. Manufacturing time
  - b. Shipping time
  - c. Interface with other materials
    - 1. Engineering judgments



- d. Storage Requirements
- e. Specific timing for installation
- f. Specific equipment needs for handling materials
- g. Environmental requirements for all materials
- h. Protection requirements during shipping of all materials
- i. Verification of shipped materials match reviewed submittals
- **2. SUBSTRATE:** All installations must accept the previous work upon which they are built. Factors affecting construction and/or installation are tracked diligently. These factors include:
  - a. Condition of materials prior to installation
  - b. Installed environmental requirements of all materials
  - c. Know the tolerances of all work on which a material is dependent (dimensional, moisture, other)
  - d. Know the pre-installation preparation of all materials

#### 3. INSTALLATION:

- A. Review Mock-up and Mock-up notes as appropriate
- B. Specific installation equipment required
- C. Understanding of protection of adjacent surfaces and environment
- D. Quantity of material needed on an as-needed basis
- E. Engage Manufacturer's representatives as needed
  - 1. Site visits
  - 2. Reports
  - 3. Updated data which may not be in original submittals
- 9. ADOPT BEST PROCEDURES: BB Incorporated will use the most appropriate procedures for all construction as required by the schedule, the budget and as the quality of the materials and their integrated use with one another demands. We work continuously with a "Zero Punch List". This list tracks ongoing corrections to work as required. We do this to eliminate a build-up of tasks to be performed at the end of the project and it furthers an ongoing consciousness of Quality.
  - 1. Review all specification sections and contact all governing agencies and associations representing all aspects of the work.
  - 2. installation, protection and maintenance of all work.



- 3. Include **Owner** and the entire design team and all members of the project in all decisions which are not directly defined by the contract documents and are not defined as "means and methods."
- 4. Maintain an ongoing consciousness of Quality
- 5. Understand all the tolerances of all installations and materials.
- 6. Utilize experts in the field
- 7. Mock-up complex constructions NOTE: Make all corrections to mock-ups prior to actual installation of representative work and get sign-off
- 8. Maintain a clean site

#### 10. SUBMITTALS AND MOCK-UPS

The SUBMITTAL and MOCK-UP processes are key components to the overall success of the project. All subcontractors must submit and receive approval for all submittals prior to commencement of any and all work. Submittals may include, but are not limited to, the following items:

#### A. SUBMITTALS

- 1 Product Data
- 2. Shop Drawings
- 3. Samples
- 4. Coordination Drawing Submittals
- 5. Test and Inspection Reports
- 6. Green Roads required documentation
- 7. Qualification Data
- 8. Welding Certificates
- 9. Installer Certificates
- 10 Product Certificates
- 11. Material Certificates
- 12. Material Test Reports
- 13. Product Test Reports
- 14. Compatibility Test Report
- 15. Field Test Reports
- 16. Design Data

#### B. MOCK-UPS

Mockups are constructed to confirm selections made under sample submittals; to demonstrate aesthetic requirements, qualities of materials and workmanship; to review coordination, testing or operation; to show interface between dissimilar materials; and to demonstrate compliance with specified installation



tolerances. Mockups are not samples. Unless otherwise indicated, approved mockups establish the standard by which work will be judged and accepted. Mockups must be approved by the architect and client prior to installation of all related work. All mockups approved must be relayed to construction and sub-teams at an appropriate time.

#### **MOCK-UPS** may include:

- 1. Integrated Exterior Bridge Mockups: Mockups of the exterior envelope erected separately from the bridge, which may consist of multiple products, assemblies, and subassemblies.
- 2. Interior Bridge Mockups: Mockups of typical interior spaces, which may include median dividing sections, pavement, and twin towers, cable systems, specialties, equipment, and lighting.
- 3. Visual Mockups: Full size physical assemblies constructed with specified materials and finishes which are singularly provided for aesthetic review and not subject to any testing regime. The Visual Mockup(s) may be required before, after or during mockup evaluation.

#### C. QUALITY BENCHMARKS:

- 1. Erect complete sections of elements of the works, as described in the Specifications, for approval. Installations must not begin in other areas of that particular trade until the design team and owner have examined and approved the quality benchmark. Alterations and adjustments required by the design team and/or client must be promptly implemented.
- 2. Upon receipt of the design team and client approval, fully protect the Quality Benchmark. The mock-up and its associated Quality Benchmarks will be used as the standard of construction and materials for the remaining work. Coordinate observation of "Quality Benchmarks" as requested by the design team, the client and any subcontractors as necessary. Remove and replace protection as necessary to facilitate observations.

#### D. SUBMITTAL REGISTER

The **BB Incorporated** Project Engineer is responsible for extracting the submittal requirements from the contract documents and preparing a "**Submittal Register**" for every specification section of the work. The **BB Incorporated** Project Engineer will maintain this record of the status of all submittals. The Submittal Register must include both the Technical Specifications and the General Requirement Specifications submittals. **EVERY SUBCONTRACTOR MUST SIGN OFF ON THIS REGISTER.** The Submittal Register must include the following:

- 1. Scheduled data for first submittals.
- 2. Specification Section number and title.
- 3. Name of subcontractor.
- 4. Description of Work covered.



- 5. Scheduled date for design team approval.
- 6. Scheduled date of fabrication.
- 7. Scheduled dates for purchasing.
- 8. Scheduled dates for installation.
- 9. Activity or event number.

#### C. SUBMITTAL APPROVAL PROCESS

The progression of submittals from transmittal to approval will be as follows:

- 1. Enter subcontractor submittal into the Submittal Register
- 2. Review the submittal package for conformance with the Contract
- 3. Verify the subcontract or purchase order for the scope of work.
- 4. Coordinate shop drawings with associated trades.
- 5. Check for rough-in requirements of other trades including such items as: Embedment in concrete pours or additional supports required attached to the steel structure.
- 6. Coordinate details, sections, dimensions, and cross-referencing to other components of work

#### F. SUBSTITUTION REQUEST

If the submittal deviates from the Contract Documents, the subcontractor must follow the contract documents for a full Substitution Request. The team reviews this request to ascertain the reasons and the appropriateness of the request. Only after The Team agrees the Substitution Request is necessary will this procedure be utilized.

#### 1.2 Quality Control

**Quality Control** assures the planning is realized in the built environment. This includes First-In-Place reviews as these are intended to be a part of the final product. We understand the final goal of this project is the realization in the built environment of all of the critical success factors. The method employed to ensure this is accomplished is Quality Control. Armed with a clear expectation, BB Incorporated uses the following tools to progress the work to its successful conclusion:

- 1. FIRST IN PLACE
- 2. REPORTING
- 4. INSPECTIONS AND TESTS
- 5. NON CONFORMANCE AND CORRECTIVE ACTIONS
- 6. QUALITY AUDITS



#### 7. RECORD AND DOCUMENT CONTROL

#### 1. FIRST-IN-PLACE

"First-In-Place" (FIP) work is a body of work which will serve as an example of the quality expected in all subsequent, similar work. The definition of all individual "First-In-Place" work and the list of all "First-In-Place Reviews" is agreed upon in the "Quality Kick-off Workshop." Upon "completion" of construction of a "First-In-Place", BB Incorporated will observe the work to ensure conformity with the contract documents and the "Quality Kick-off Workshop" expectations. After a thorough review, including all tests and inspections as required by the contract documents, all "in-house" deficiencies are documented in a "Non-Conformance Report" form. These forms become part of the 3-week look-ahead schedule and are tracked in V-Planner and are continually reviewed until cleared at the weekly OAC meetings. All First-In-Place activities and schedules will be discussed in the Quality Workshops. Upon correction of ALL items for any defined "First-In-Place," appropriate team members will be invited to review corrected work. Again, all found deficiencies are attached to the 3-week look-ahead schedule and are tracked in subsequent OAC and Quality Workshop meetings. The client and/or the design team are then asked to conduct a final "First-In-Place Sign-off." This work then serves as the example for all subsequent, similar work.

If, in the unlikely event the "First-In-Place" work cannot meet the team's quality expectations a "Root Cause Analysis (RCA)" will be conducted. This analysis determines reasons and corrective actions to be conducted to achieve the required success on the specific "First-In-Place" work. The learned resolution is then implemented in the correction of the "First-in-place" and an additional "final inspection" is called. When this "final inspection" is approved, all further work will conform to the level achieved and finally agreed upon.

#### A. ROOT CAUSE ANALYSIS (RCA)

The "Root Cause Analysis" is a tool which diagnoses reasons the work was difficult to perform or reasons the work could not meet all Quality expectations. This analysis provides "lessons learned" to inform all future work.

#### B. NON CONFORMANCE REPORT (NCR)

A "Non Conformance Report" will document clearly nonconforming work found by test or quality inspection or review. All Non Conformance Reports will be logged and tracked through a "Continuous Improvement Plan Log". It is the first step of the "Corrective Action Plan". The NCR log will be maintained by BB Incorporated and kept in Egnyte and hardcopy.



#### C. CORRECTIVE ACTION PLAN (CAP)

A "Corrective Action Plan" includes all "Non Conformance Reports" and all "Root Cause Analysis" for a specific "First in Place". The CAP provides one of 2 paths forward:

- 1. Identify methods to achieve the agreed upon objectives (re: "Quality Kick-off Workshop")
  - a. Describe in detail work methods
  - b. Identify all changes in the schedule
  - c. Identify all changes in the project costs
- 2. Clearly state why the objectives cannot be met
  - a. Describe in detail alternate, equivalent objectives
  - b. Describe in detail alternate methods to achieve equivalent objectives
  - c. Identify all changes in schedule
  - d. Identify all changes in project costs

#### 2. REPORTING

#### A. DAILY REPORTING

BB Incorporated is responsible for all "Daily Reporting". This includes:

- 1. Tracking all Subcontractors' work
- 2. Tracking all activities completed
- 3. Observing and reporting on all work in progress
- 4. Documenting all problems encountered and action taken
- 5. Recording all tests and inspections performed; indicate any failed tests or inspections, and steps taken
- 6. Identification of any non-conforming work
- 7. Follow-up inspections on corrective work

#### B. MONTHLY REPORTING

BB Incorporated will submit a monthly Quality Control Report prepared by the **BB** Incorporated Quality Lead which will be included as part of an OAC meeting once a month to discuss. This report includes:

- 1. A summary of all work completed and in progress
- 2. All outstanding quality issues
- 3. Tracking all Subcontractors' work
- 4. Tracking all activities completed



- 5. Observing and reporting on all work in progress
- 6. Documenting all problems encountered and action taken
- 7. Recording all tests and inspections performed; indicate any failed tests or inspections, and steps taken
- 8. Identification of any non-conforming work
- 9. Follow-up inspections on corrective work

BB Incorporated holds all subcontractors responsible for their own weekly reports which include:

- 1. A summary of all work completed and in progress
- 2. All outstanding quality issues
- 3. Tracking all activities completed
- 4. Observing and reporting on all work in progress
- 5. Documenting all problems encountered and action taken
- 6. Recording all tests and inspections performed; indicate any failed tests or inspections, and steps taken

#### 3. INSPECTIONS AND TESTS

Inspections and tests are necessary to verify work processes and results conform to contract requirements and to the mutual understandings as defined in the "Quality Kick-off Workshop". Parties performing inspections and testing include but are not limited to: BB Incorporated, client, design team design consultants, third-party inspection and testing agencies, governing agencies, and subcontractor's inspection and testing companies.

Each subcontractor as part of their Quality Control Plan will submit an internal Inspection and Testing Plan. This internal plan must include the following items:

- 1. List of items to be inspected and tested with type of test required
- 2. Inspections and tests to be performed
- 3. Testing schedule frequency
- 4. Specification references
- 5. Performing party
- 6. Witness parties
- 7. Certificates required
- 8. Checklists and procedures
- 9. Reference standards
- 10. Acceptance criteria
- 11. Witness and/or Hold for Engineer's approval



#### A. TYPES OF INSPECTIONS AND TESTS

#### 1. Material Inspections and Testing

When required by the specifications, material quality inspections and tests ensure purchased materials meet quality requirements. The subcontractor Quality Control Manager inspects or ensures qualified inspectors inspect materials prior to use for conformance to project quality requirements.

The subcontractor Quality Control Manager is responsible for limiting all installed materials to those which have been documented as meeting the testing requirements of all specifications.

#### 2. Special Tests and Inspections

BB Incorporated will engage qualified Special Inspectors as necessary to conduct special tests and inspections. The Special Inspectors will be: **Not Yet Hired** 

- a. Verify manufacturers maintain detailed fabrication and quality-control procedures
- b. Review the completeness and adequacy of the procedure as it relates to the Work.
- c. Notify BB Incorporated promptly of irregularities and deficiencies observed in the work
- d. Submit a certified written report of each test, inspection, and similar quality control service to the BB Incorporated and authorities having jurisdiction.
- e. Submit a final report of special tests and inspections at Substantial Completion, which includes a list of unresolved deficiencies.
- f. Interpret tests and inspections and communicate whether tested and inspected work complies with or deviates from the Contract Documents.
- g. Retest and re-inspect the corrected work.

### 3. Source quality inspections:

- a. Required when quality characteristics cannot or will not be verified during subsequent processing.
  - b. Validate supplier quality before materials are delivered to the project jobsite.
- c. Ensure each work task uses the source inspected materials and proceeds only after materials have been accepted by the source inspection.
  - d. Submit source inspection and testing reports.

#### 4. Work In-progress Inspections

"Work in-process" quality inspections verify compliance to project quality standards as work is being conducted. Work in-progress inspections are conducted by the subcontractor



Quality Control Lead and submitted to BB Incorporated and are discussed in the OAC Weekly Meetings.

## 5. Initial Job-Ready Inspections

For each work task, the project team performs job-ready quality inspections to ensure that work activities begin at the appropriate time. Job-ready quality inspections verify starting conditions conform to the project quality requirements.

#### 6. Initial Work In-Progress Inspections

For each work task, the project Team performs an initial work in process inspection when the first representative portion of a work activity is completed to assure conformance with contract documents and expectations as discussed in the "Quality Kick-off Workshop."

#### 7. Follow-up Work In-Process Inspections

**BB Incorporated** performs ongoing work in process quality inspections to ensure work activities continue to conform to project quality requirements.

#### 8. Completion Work Quality Inspections

For each work task, the project team inspects the completed work to verify work conforms to the project quality requirements. The **BB Incorporated** Project Manager, must review added requirements as requested by the owner for schedule and budget analysis and agreed upon in the "Quality Kick-off Workshop". Appropriate change orders must be processed prior to the implementation of new requirements.

"Completion Quality Inspections" must be conducted prior to other work activities which may interfere with an inspection.

#### 9. **Product Testing**

All product testing will be performed in accordance with project specifications.

#### 10. Field Quality Control Testing

This includes tests and inspections performed on-site for installation of the Work and for completed Work.

**5. NONCONFORMANCE AND CORRECTIVE ACTIONS:** A nonconformance is any item or assembly which does not meet project quality requirements. Should a nonconformance item or assembly be identified by an inspection or test, a systematic method to identify the item, correct it, and ensure project quality is not adversely impacted must be followed.



#### A. OBSERVATION AND MARKING OF NONCONFORMANCE WORK

When BB Incoporated, subcontractors/suppliers, inspectors, or the client identify a nonconformance item, the effected subcontractor must promptly and clearly mark by paint, tape, tag, or other easily observable signal the area, part, adjacent area, affected areas, adjacent parts, to prevent the inadvertent cover-up of all work and all work affected by the nonconforming work.

#### **B. CONTINUATION OF WORK**

After the "nonconformance" is marked, the **BB Incorporated Project Manager** determines if work can continue in the affected area. Work may continue if:

- 1. Continuation of work in the area does not adversely affect quality or hide the defect
- 2. Continuation of work does not affect the quality of adjacent or hide any and all impacts on adjacent work

Work may continue in the affected area while the disposition of the item is resolved. The BB Incorporated Project Manager, may place limitations on the continuation of work. All limitations must be communicated as quickly as possible and be followed up in writing.

Note: All work whether or not it is determined by the BB Incorporated Project Manager to be continued or not will be documented on the NCR Log. All limitations of continuing work will be included in the Non-Conformance Log.

- **C. STOP WORK ORDER:** The **BB Incorporated Project Manager**, may stop work if ANY of the following conditions occur:
  - 1. Continuing work may adversely affect the project quality in any way
  - 2. Continuing work may hide defective work
  - 3. Defective work has not been adequately documented
  - **4.** Defective work has not been clearly marked
  - 5. The area affected by the defective work has not been clearly marked

The Project Manager identifies the limits of the affected area and quickly and clearly marks the stop work area and communicates in writing to all those affected.

**D. DOCUMENTING NONCONFORMING WORK:** When a nonconformance item is identified, the **BB Incorporated** Quality Lead, must notify the subcontractor's Quality Control Manager as soon as practical. This notification must be followed-up in writing. The subcontractor's Quality Control Manager must issue a Nonconformance Report and log and track the item until the nonconformance item has been corrected and approved. The **BB Incorporated** Quality Lead will document all nonconforming work.



- E. DISPOSITION OF NON-CONFORMANCE: When the subcontractor's Quality Control Manager is notified of a nonconforming item, the subcontractor Quality Control Manager must complete a "Nonconformance Report Form" and be entered into the Non-Conformance Log. The subcontractor's Quality Control Manager must assign a disposition of one of the following:
  - 1. **REPLACE:** The nonconformance may be brought into conformance with the original specification requirements by replacing the nonconforming product or material with a conforming product or material.
  - 2. **REPAIR:** The nonconformance may be brought into conformance with the original requirements through re-machining, reassembly, reprocessing, reinstallation, or completion of the required operations. Major repair requires **Owner**, the design team and BB Incorporated's written approval prior to rework.
  - 3. **REWORK:** The nonconformance may be made acceptable for its intended use, even if it is not restored to a condition meeting all specification requirements. The subcontractor Quality Control Manager may specify applicable standards for the completion of rework. Such standards and rework must be approved by **Owner**, the and BB Incorporated and all appropriate governing agencies.

The subcontractor's Quality Control Manager will complete the NCR form; identifying the non-conforming work and the recommended disposition. The NCR will be given to **Owner**, BB Incorporated and all appropriate governing agencies for a full review. If the Project Team agrees with the subcontractor Quality Control Manager's recommendation, the **BB Incorporated Project Manager** and **Owner** will approve and work may commence. No work specifically pertaining to the non-conformance item(s) is to occur until the disposition/recommendation has been approved by the Project Team.

#### F. CORRECTION OF NONCONFORMING WORK

The **BB** Incorporated **Project Manager** must verify corrective actions have eliminated the nonconformance to the requirements of the contract documents, or as instructed by the disposition of the nonconformance report, and only then removes the nonconformance marker. It is the sole responsibility of the BB Incorporated Project Manager (or other BB Incorporated designee) to remove the nonconformance marker.

#### G. LESSONS LEARNED - PREVENTATIVE ACTIONS

Fixing nonconforming work includes systematic prevention of recurrences. It is essential for improving the quality of the project that all Nonconformance issues undergo a Root Cause Analysis (RCA). The RCA will identify the quality expected for the item, versus the actual quality observed. For each item, a corrective action will be identified and implemented. Solutions may involve a combination of enhanced process controls, training, upgrading personnel qualifications, or improved processes. Follow-up is performed to verify the problem is resolved.



#### 6. OUALITY AUDITS

Audits verify the BB Incorporated Quality Plan is functioning as intended. The Project Team conducts Project Quality audits every three months to verify proper operation of the Quality Plan. The Project Team will audit:

- 1. Project—specific quality standards
- 2. Process control plans
- 3. Inspections and tests
- 4. Nonconformance and corrective actions
- 5. Preventive actions
- 6. Quality records and documents

The Project Quality Team is responsible for taking appropriate corrective actions to ensure compliance with Quality Plan requirements.

#### 7. RECORD AND DOCUMENT CONTROL

Proper document control facilitates the organized flow of documents and correspondence; promotes consistent distribution of data; promotes proper identification and storage of documents; and thus ensures that all Project participants have access to the most current information.

#### A. QUALITY SYSTEM DOCUMENTS

#### 1. Quality Plan

The **BB Incorporated** Quality Lead will maintain the **BB Incorporated** Quality Plan, and ensure approved subcontractor QCPs are in place and accessible. The Quality Manager maintains, improves, and updates the Quality Plan as necessary. Every three months, the **BB Incorporated** Quality Lead evaluates the process to assess possible improvements.

#### 2. Document Controls

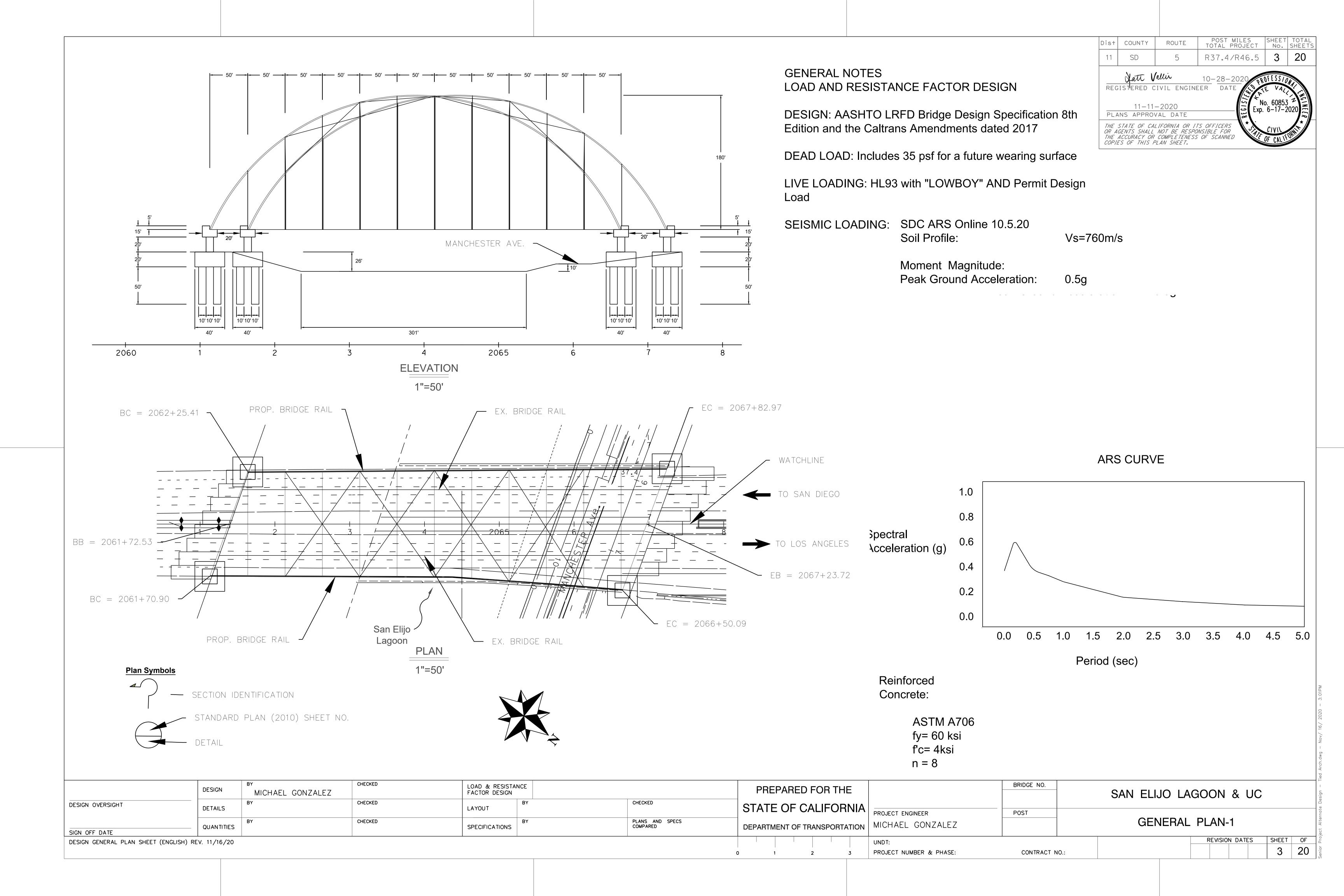
Document Control maintains project-specific quality documents; ensures applicable documents are readily available; and seeks to eliminate unintended use of obsolete documents.

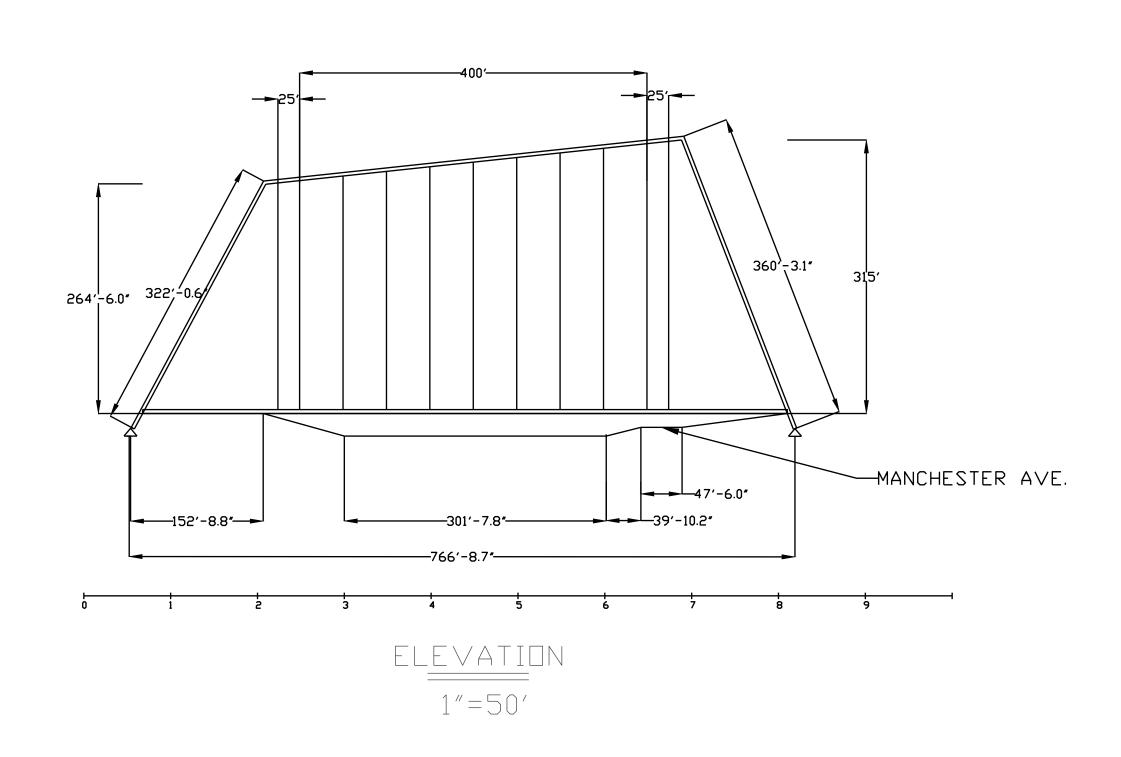
#### 3. Project Controls Record

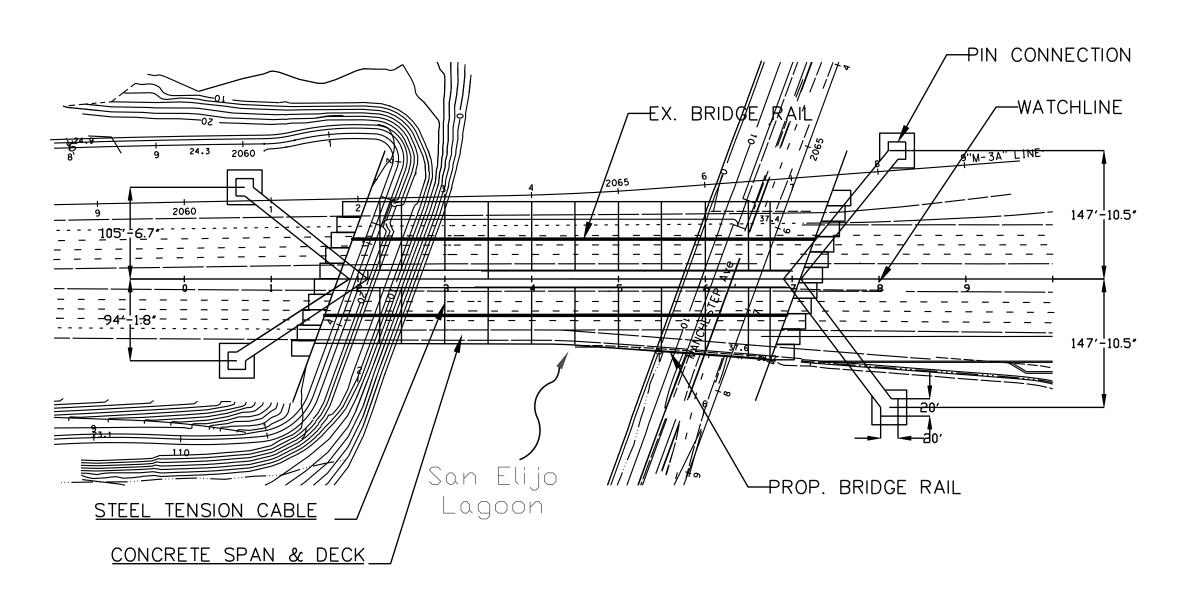
The **BB Incorporated** Quality Manager verifies the completeness, accuracy, and retention of project specific quality records including:

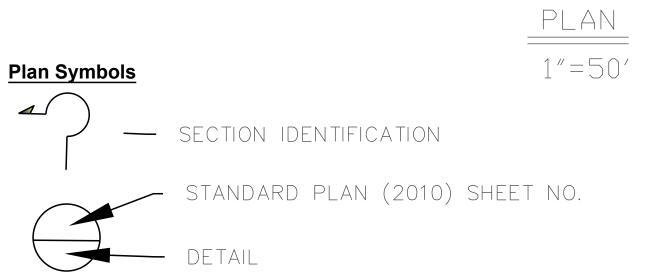
- 1. Inspection and test records
- 2. Quality submittals to owner and design team
- 3. Field reviews
- 4. Daily log reports
- 5. Incident reports
- 6. Subcontractor Quality Control Plans
- 7. Quality improvement records
- 8. Nonconformance reports

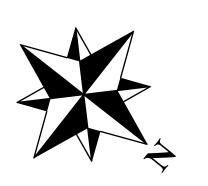
# Alternative Technical Concpets











**GENERAL NOTES** LOAD AND RESISTANCE FACTOR DESIGN

DESIGN: AASHTO LRFD Bridge Design Specification 8th Edition and the Caltrans Amendments dated 2017

DEAD LOAD: Includes 35 psf for a future wearing surface

LIVE LOADING: HL93 with "LOWBOY" AND Permit Design Load

SEISMIC LOADING: SDC ARS Online 10.5.20

Soil Profile:

Vs=760m/s

Moment Magnitude:

Peak Ground Acceleration: 0.5g

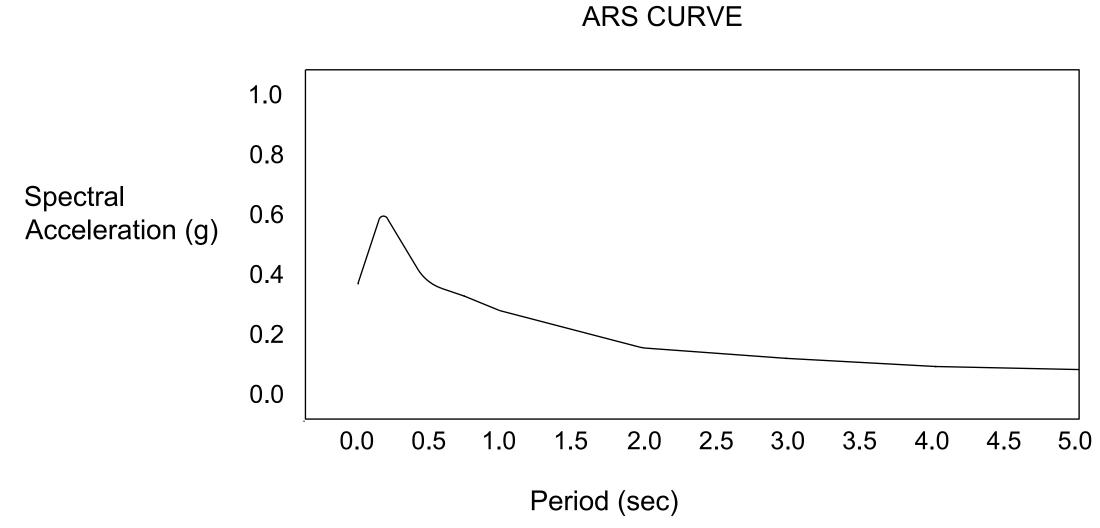
POST MILES SHEET TOTAL TOTAL PROJECT No. SHEETS Dist COUNTY ROUTE R37.4/R46.5 SD Hatt Vallin

REGISTERED CIVIL ENGINEER DATE

11-11-2020 Plans approval date

THE STATE OF CALIFORNIA OR ITS OFFICERS
OR AGENTS SHALL NOT BE RESPONSIBLE FOR
THE ACCURACY OR COMPLETENESS OF SCANNED
COPIES OF THIS PLAN SHEET.

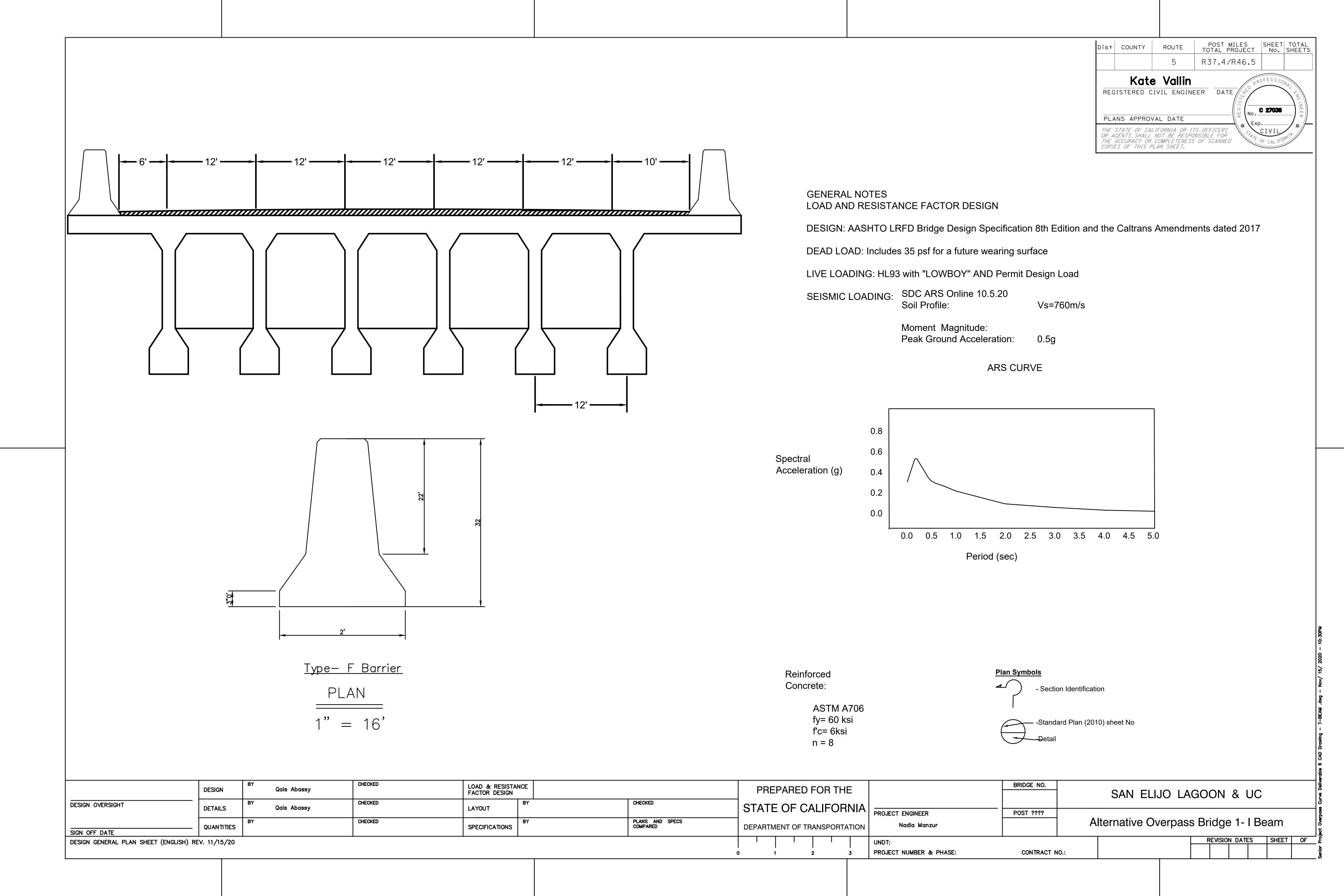




Reinforced Concrete:

> ASTM A706 fy= 60 ksi f'c= 4ksi n = 8

	DESIGN	HOWARD LAU	CHECKED	LOAD & RESISTA FACTOR DESIGN		PREPARED FOR THE	BRIDGE NO.	SAN ELIJO LAGOON & UC
DESIGN OVERSIGHT	DETAILS	BY	CHECKED	LAYOUT	BY CHECKED	STATE OF CALIFORNIA PROJECT ENGINEER	POST	
SIGN OFF DATE	QUANTITIES	BY	CHECKED	SPECIFICATIONS	BY PLANS AND SPECS COMPARED	DEPARTMENT OF TRANSPORTATION MICHAEL GONZALEZ		GENERAL PLAN-1
DESIGN GENERAL PLAN SHEET (ENGLISI	SH) REV. 11/16/20					UNDT: 0 1 2 3 PROJECT NUMBER & PHASE:	CONTRACT 1	REVISION DATES SHEET OF 3



GENERAL NOTES LOAD AND RESISTANCE FACTOR DESIGN

DESIGN: AASHTO LRFD Bridge Design Specification 8th Edition and the Caltrans Amendments dated 2017

DEAD LOAD: Includes 35 psf for a future wearing surface

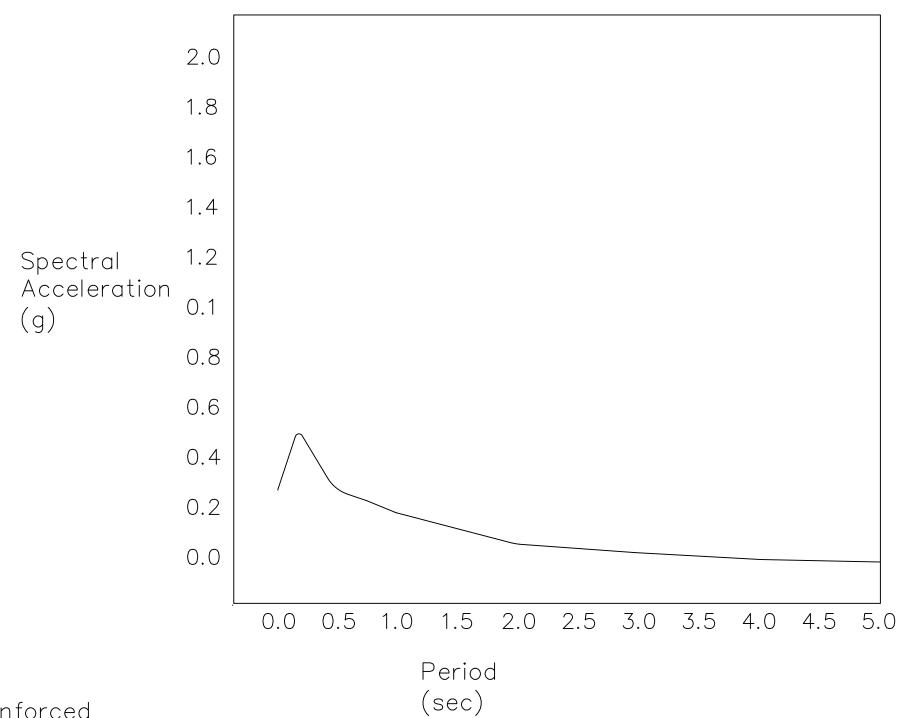
LIVE LOADING: HL93 with "LOWBOY" AND Permit Design Load

SEISMIC LOADING: SDC ARS Online 10.5.20 Soil Profile:

Vs=760m/s

Moment Magnitude: Peak Ground Acceleration: 0.5g

# ARS CURVE



Reinforced Concrete:

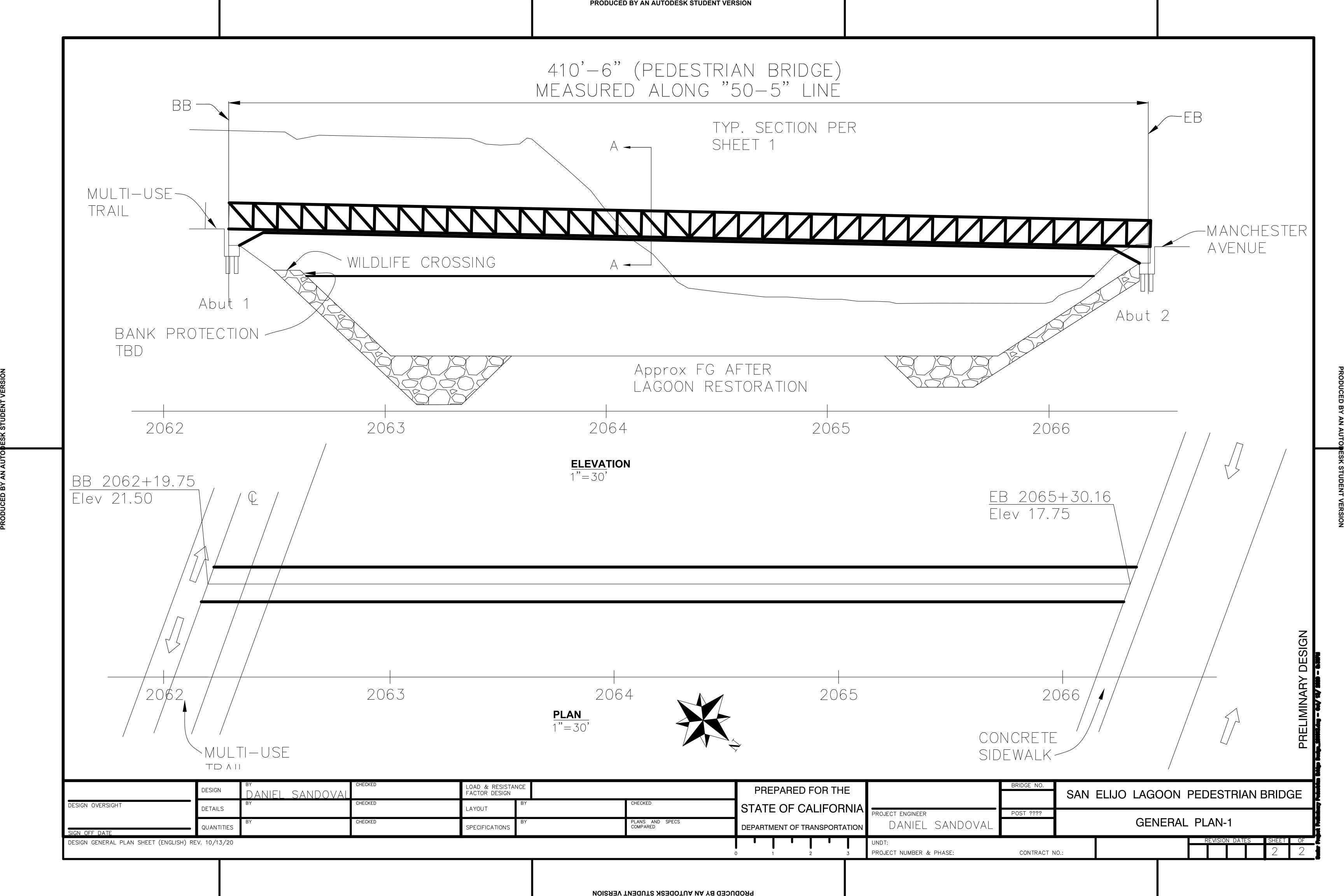
> ASTM A706 fy= 60 ksi f'c= 4ksi

n = 8

10'-0"Min CLEARANCE ENVELOPE 2%

TYPICAL SECTION A-A NTS

	DESIGN	BY Danifi sandoval	CHECKED	LOAD & RESISTA FACTOR DESIGN	NCE		PREPARED FOR THE		BRIDGE NO.	SAN ELIJO LAGOON PEDESTRIAN BRIDGE
DESIGN OVERSIGHT	DETAILS	BY	CHECKED	LAYOUT	BY	CHECKED	STATE OF CALIFORNIA	PROJECT ENGINEER	POST ????	
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DESIGN GENERAL PLAN SHEET (ENGLISH) R	EV. 10/13/20							UNDT:	CONTRACT N	REVISION DATES SHEET OF



LOAD AND RESISTANCE FACTOR DESIGN

DESIGN: AASHTO LRFD Bridge Design Specification 8th Edition and the Caltrans Amendments dated 2017

DEAD LOAD: Includes 35 psf for a future wearing surface

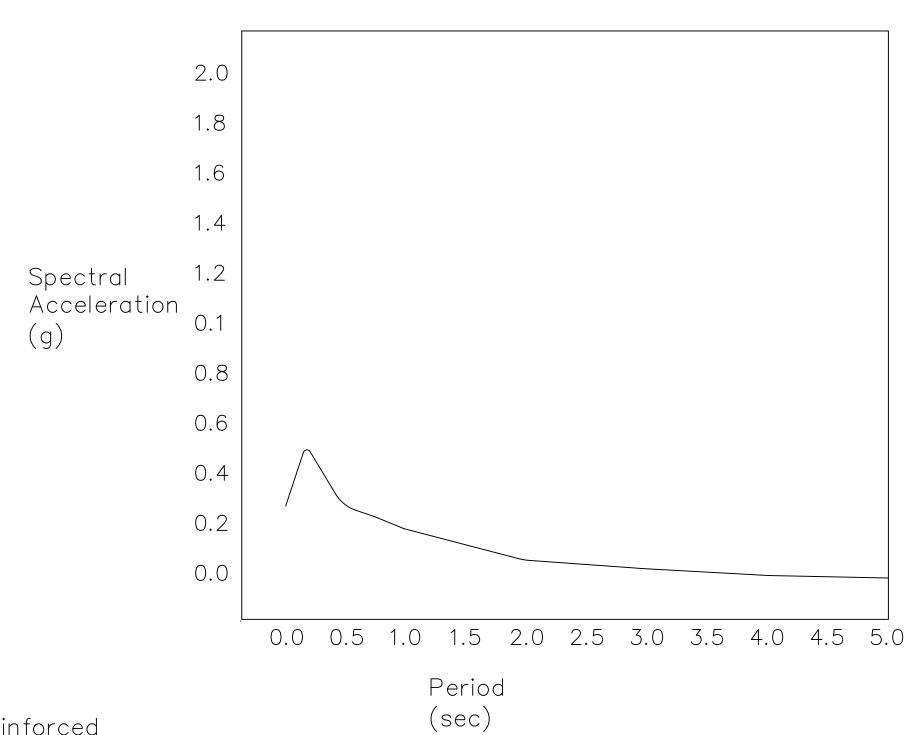
LIVE LOADING: HL93 with "LOWBOY" AND Permit Design Load

SEISMIC LOADING: SDC ARS Online 10.5.20 Soil Profile:

Vs=760m/s

Moment Magnitude: Peak Ground Acceleration: 0.5g

ARS CURVE

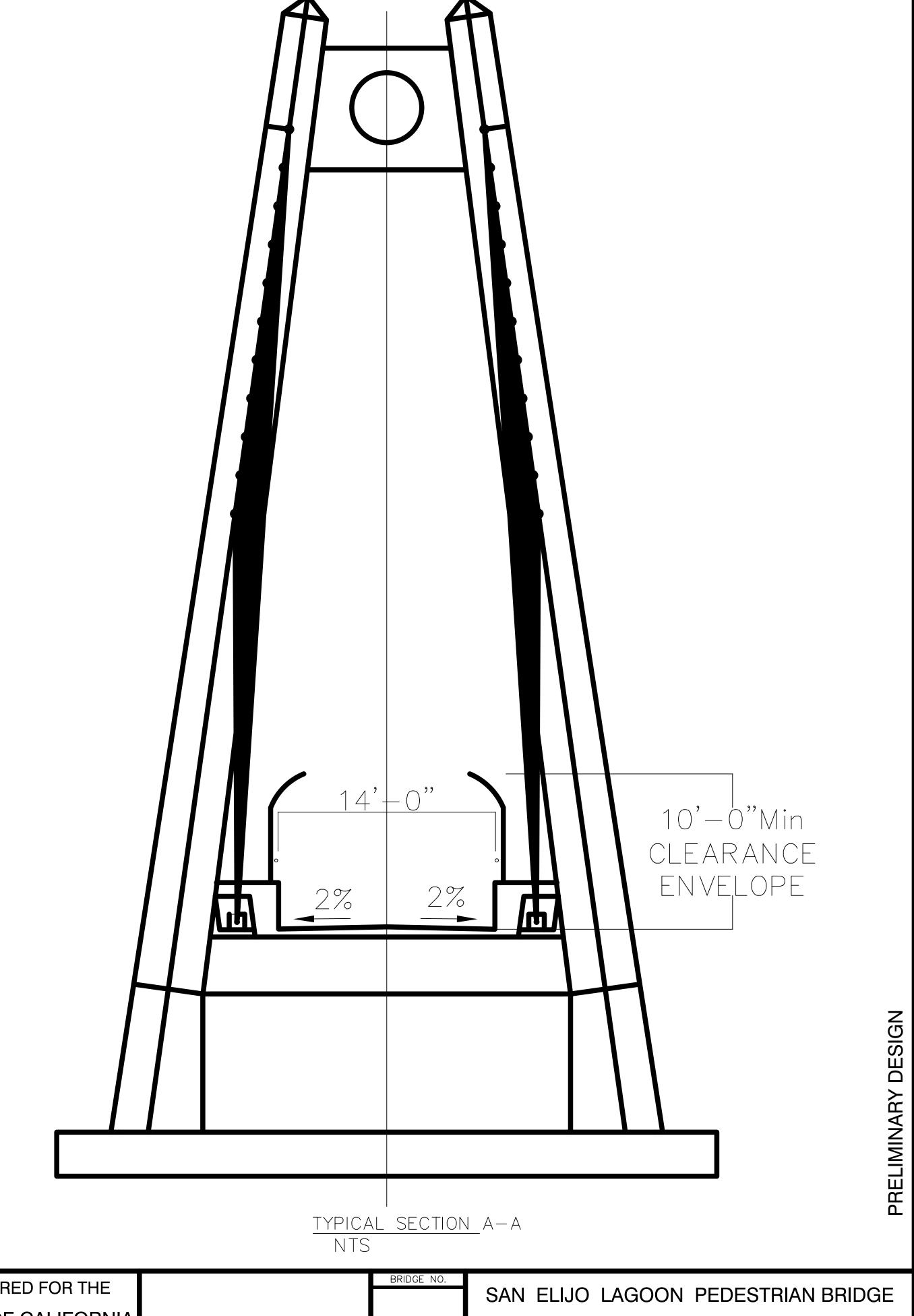


Reinforced Concrete:

ASTM A706

fy= 60 ksi f'c= 4ksi

n = 8



	DESIGN	DANIFI SANDOVA	CHECKED	LOAD & RESISTA FACTOR DESIGN	NCE		PREPARED FOR THE		BRIDGE NO.	SAN ELIJO LAGOON PEDESTRIAN BRIDGE
DESIGN OVERSIGHT	DETAILS	BY	CHECKED	LAYOUT	BY	CHECKED	STATE OF CALIFORNIA	PROJECT ENGINEER	POST ????	
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PRODUCED BY AN AUTODESK STUDENT VERSION